



**MASINDE MULIRO UNIVERSITY
OF SCIENCE AND TECHNOLOGY**

University Of Choice



**UNIVERSITÀ
DEGLI STUDI DI BARI
ALDO MORO**

Slope stability analysis: different approaches for different landslides



**UNIVERSITÀ
DI PARMA**

Prof. Roberto Valentino, PhD

*Earth Science Unit – Department of Chemistry, Life
Sciences and and Environmental Sustainability*

roberto.valentino@unipr.it

There are many problems related to LANDSLIDES.

Before going in depth, it is necessary to focus our attention on a basic issue:

Which is the main **OBJECTIVE** of my analysis?

Why I have to study this problem?

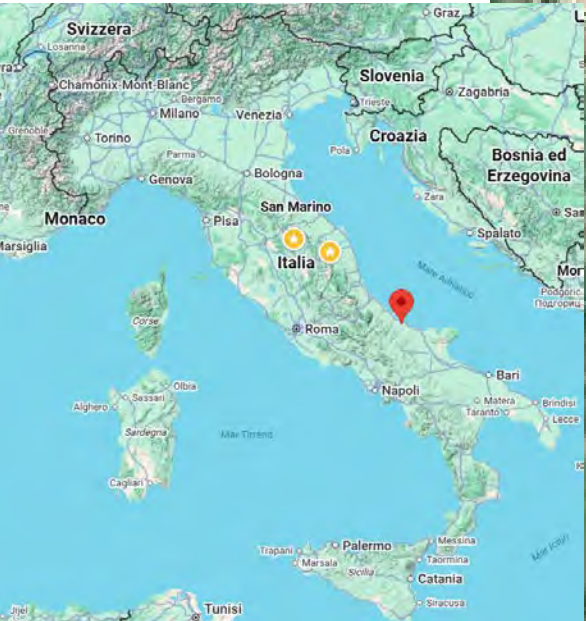
The answer to these questions determines the APPROACH and the METHODS!



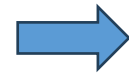
An example from Italy: Petacciato landslide



Which is the main OBJECTIVE?



An example from Kenya

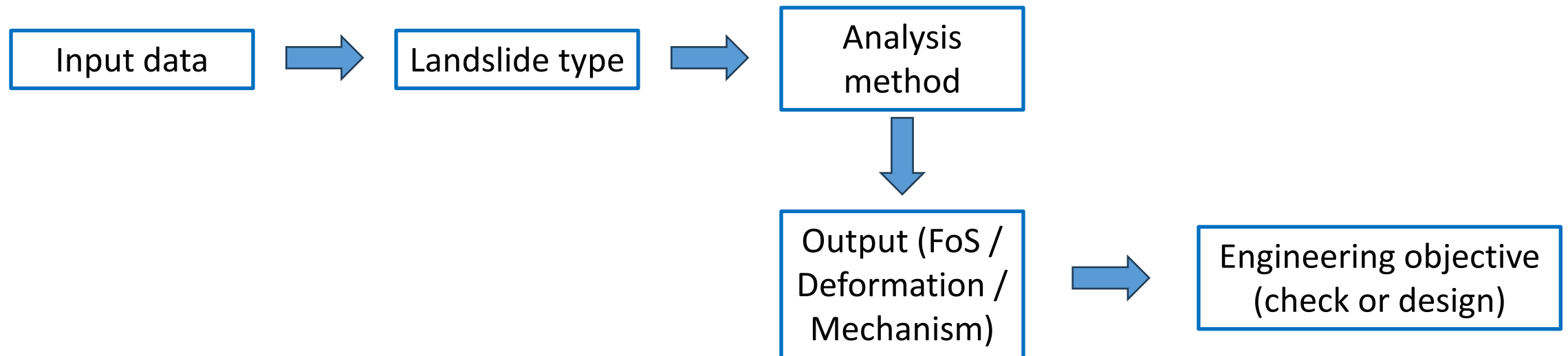


Which is the main OBJECTIVE?

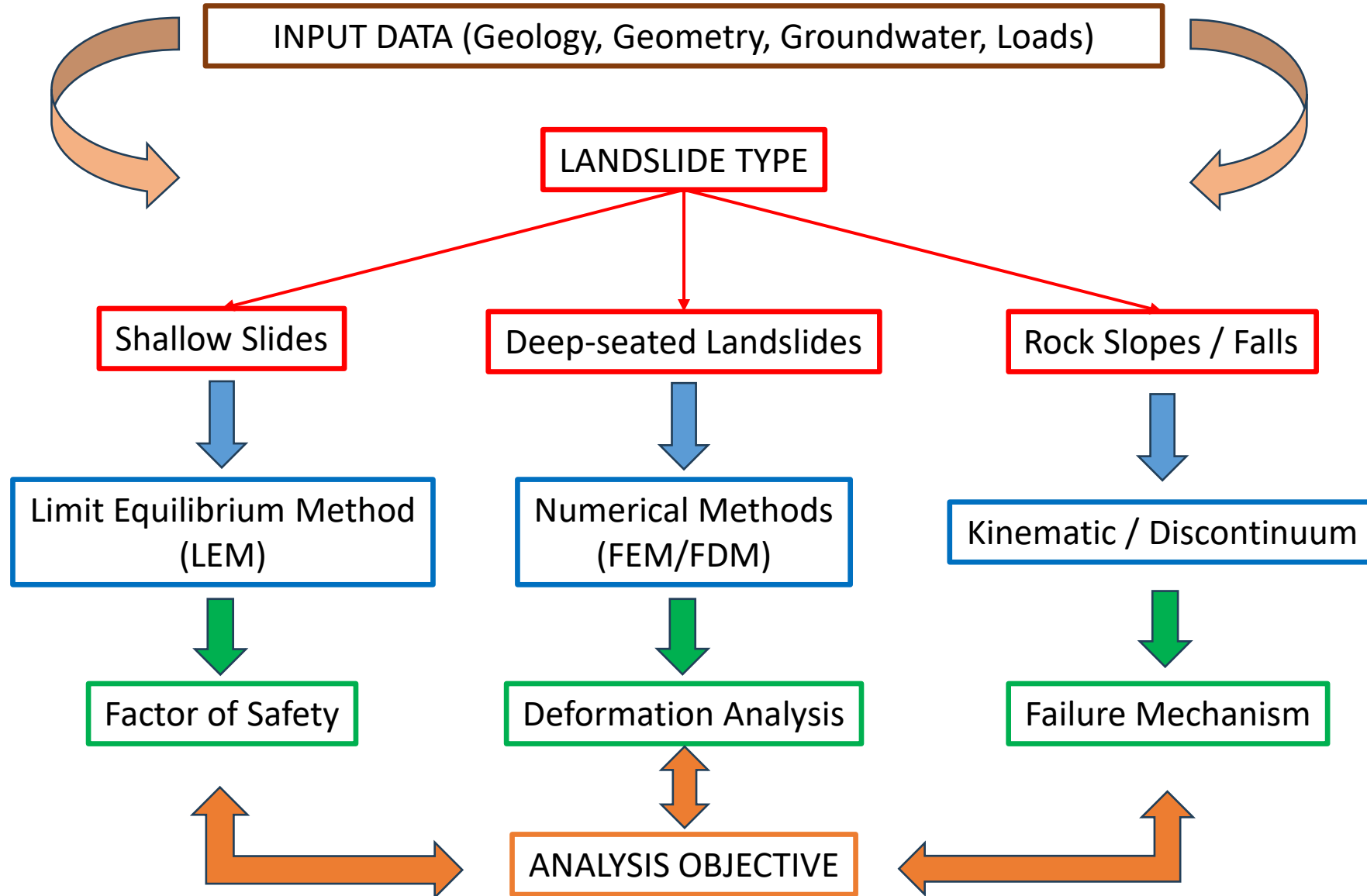


Heavy rains on November 1, 2025, triggered a massive landslide in Elgeyo Marakwet County's Marakwet East constituency, killing 21 people and leaving over 30 missing in areas like Chesongoch and Moror Village.

Different landslide types require different analysis methods, depending on the objective: from simple stability checks to detailed design and deformation analysis.



Slope Stability Analysis Framework



If the objective is stabilizing a road cut slope, we need:

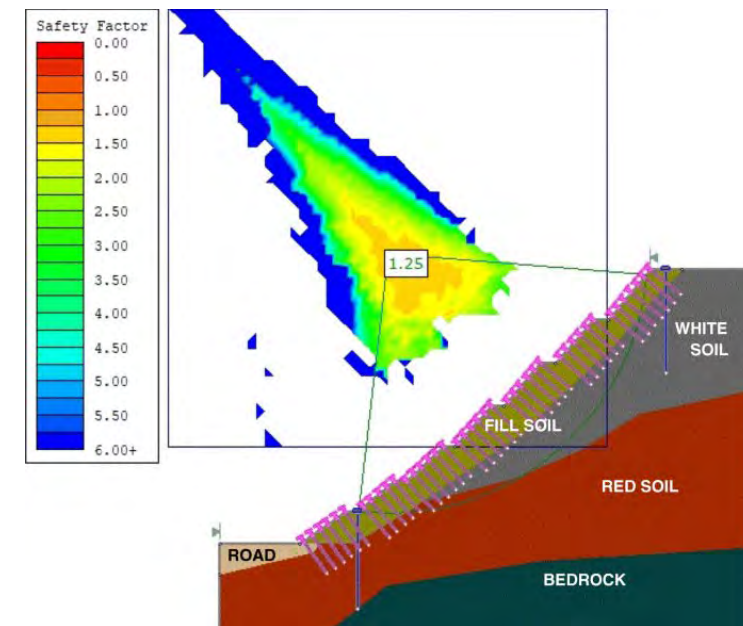
- Detailed field survey (geometry)
- Deep field (geotechnical) investigations
- Field tests
- Laboratory tests
- Numerical modeling at slope scale



(a)

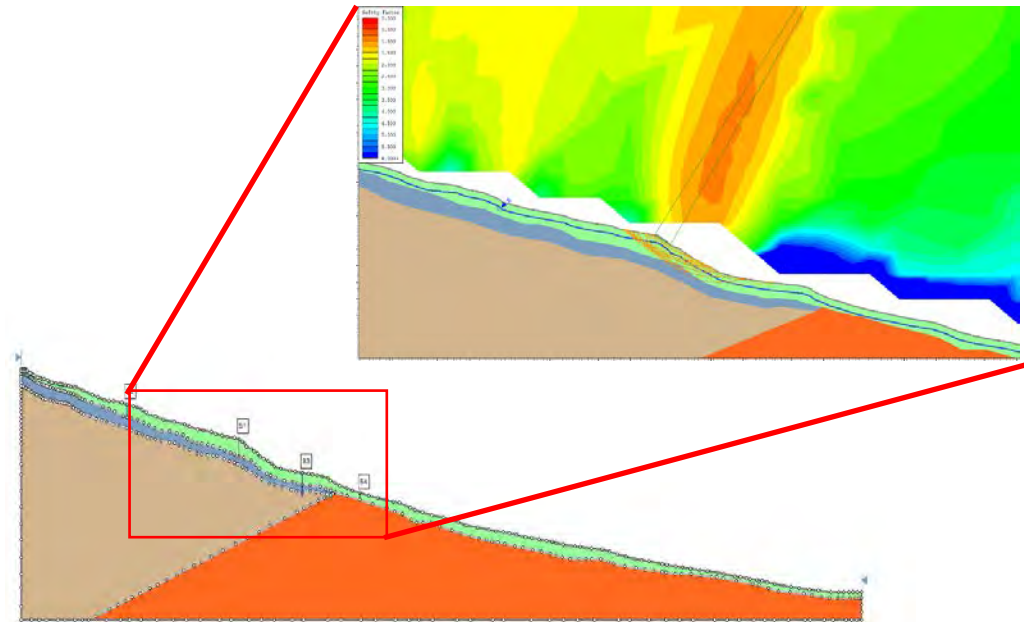


(b)



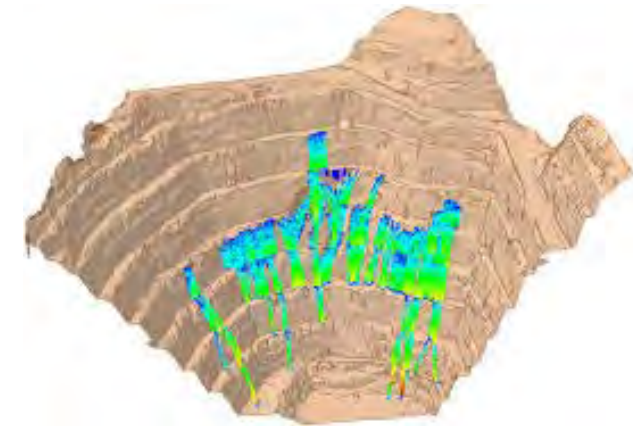
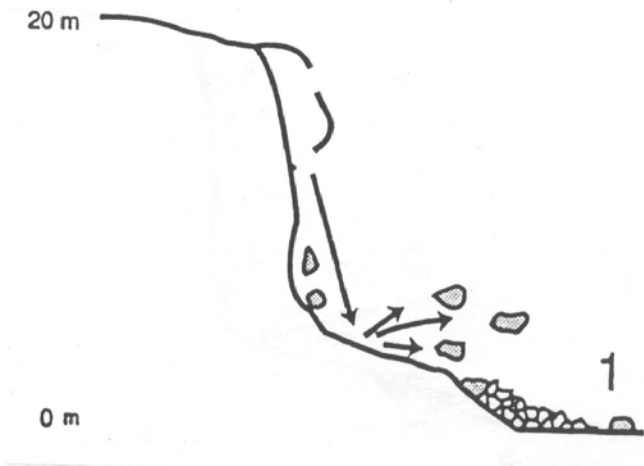
If the objective is analyzing a natural slope, we need:

- Detailed field survey (geometry)
- Deep field (geotechnical) investigations
- Field tests
- Laboratory tests
- Numerical modeling at slope scale



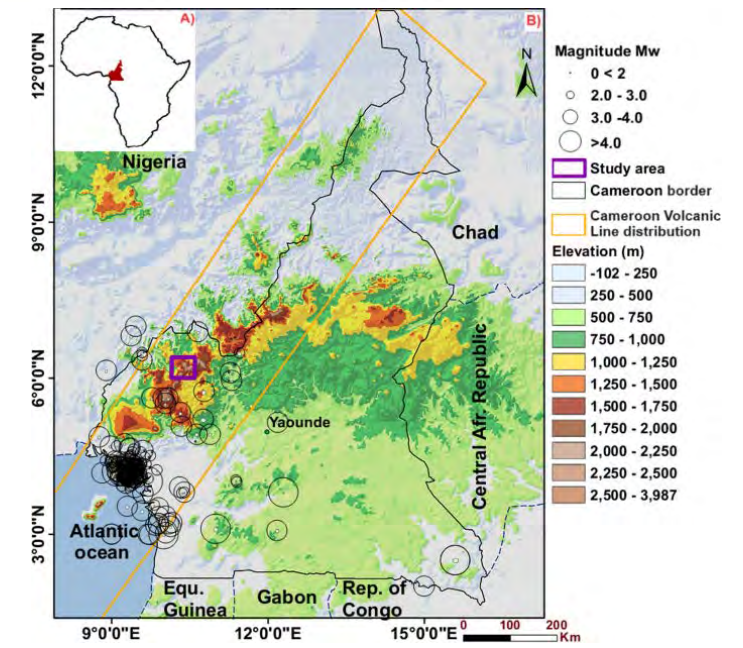
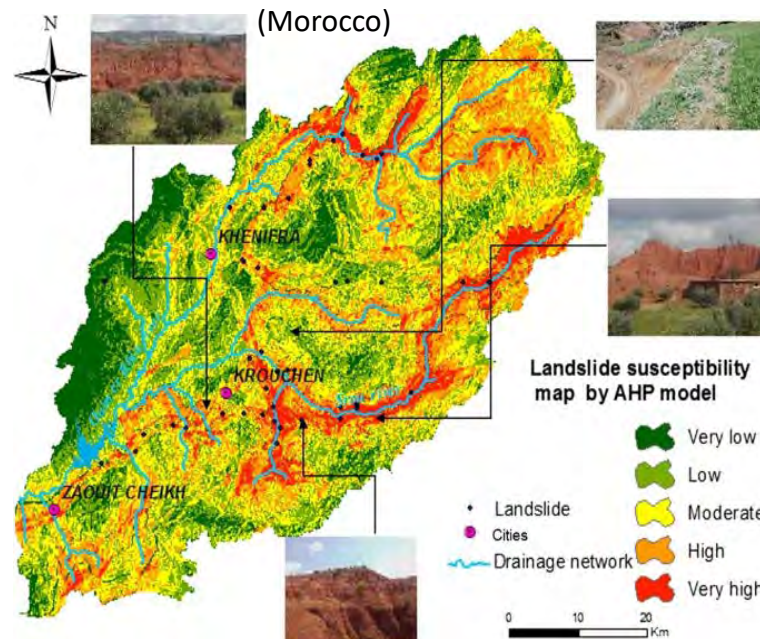
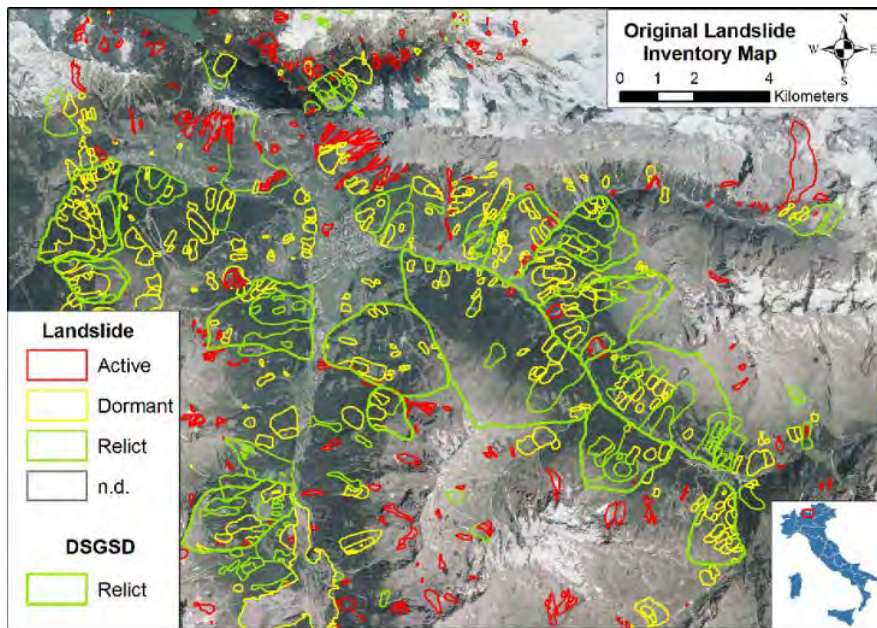
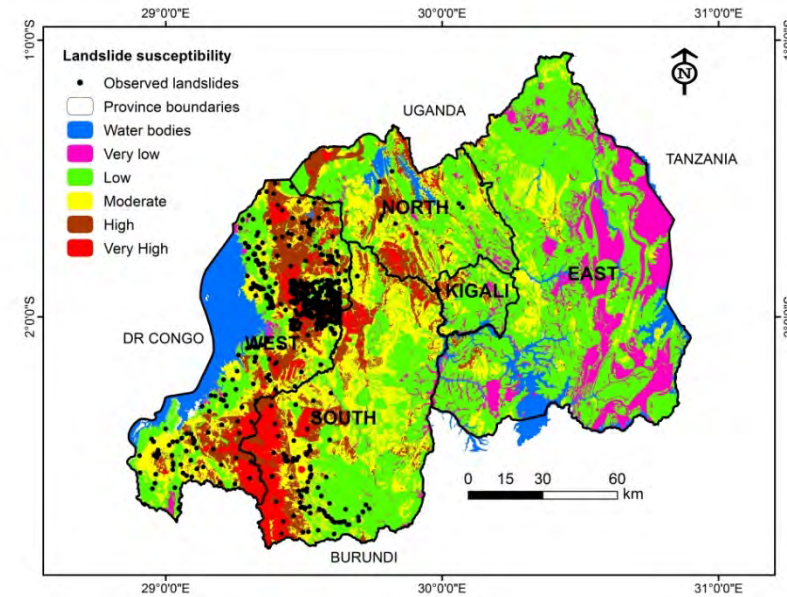
If the objective is protecting a road or an infrastructure, we need:

- Detailed field survey (geometry)
- Deep field (geotechnical) investigations
- Field tests
- Laboratory tests
- Numerical modeling at slope scale



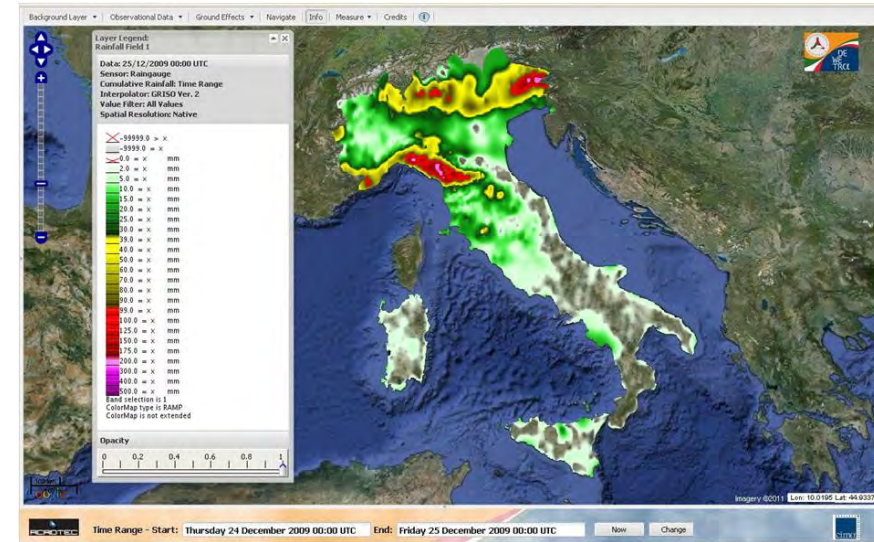
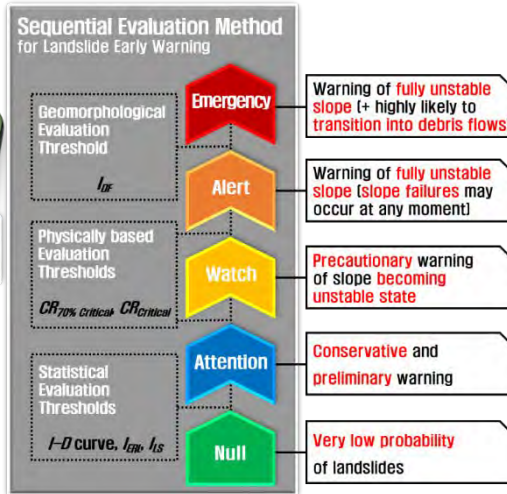
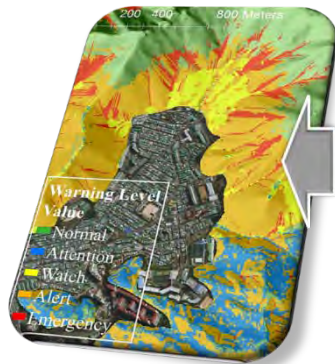
If the objective is mapping potentially unstable areas, we need:

- Field survey + aerial/satellite photos
- Inventory map
- Field (geotechnical) investigations
- Good basic datasets
- Numerical modeling at regional scale



If the objective is an early warning system...

- Field (geotechnical) investigations
- Good basic datasets
- Forecasted rainfalls
- Numerical modeling at regional scale



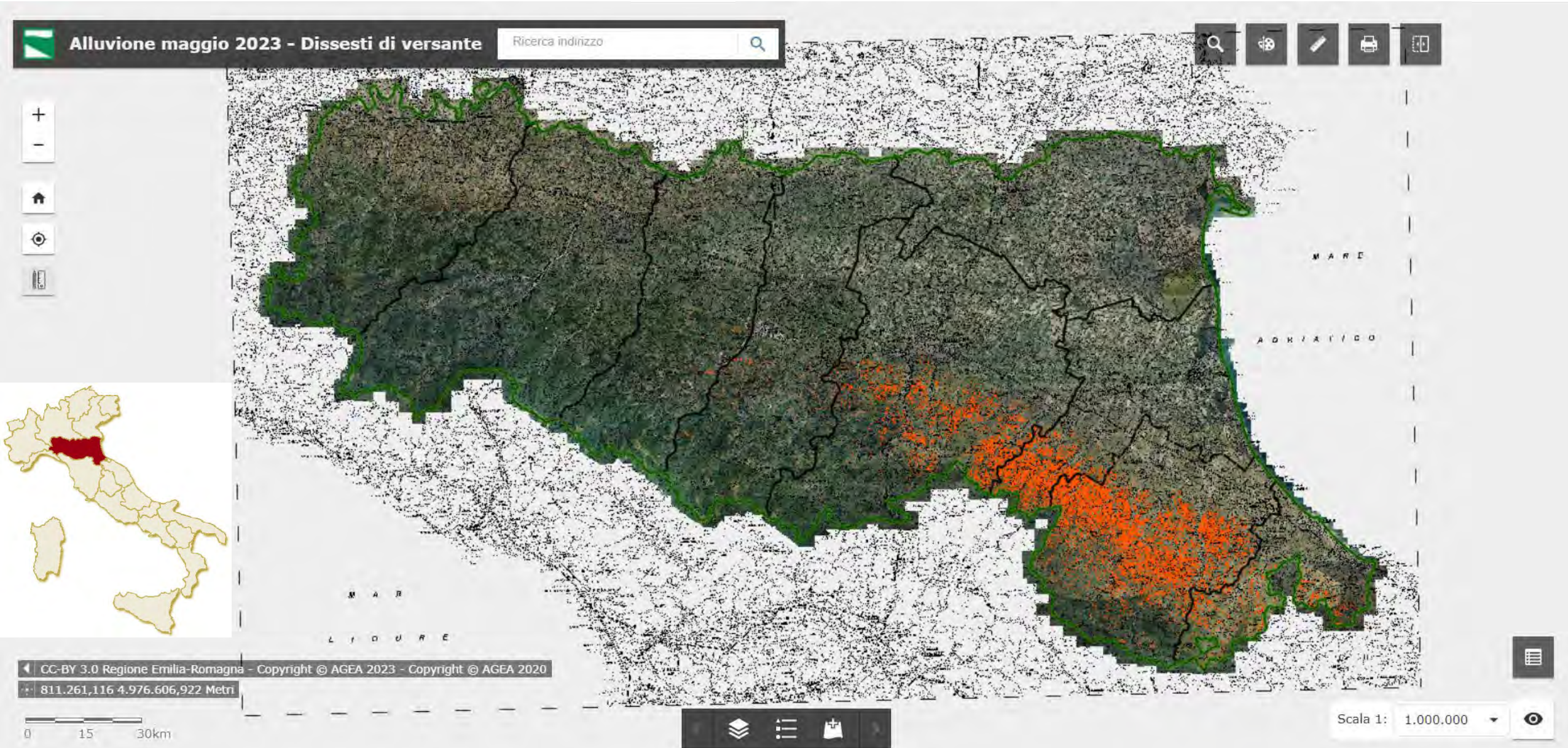
Rainfall map



Map of instability index

An example from Emilia Romagna Region...

Emilia Romagna Region, May 2023, more than 80,000 landslides...



Emilia Romagna, May 2023, more than 80,000 landslides...



Which is the objective of the regional administration?

Some problems...

...sometimes, we have results of models but NOT results of field surveys

Some questions...

What do we *really* know about the *real* phenomena?

Can we improve our knowledge?

How?

Different investigation methods...

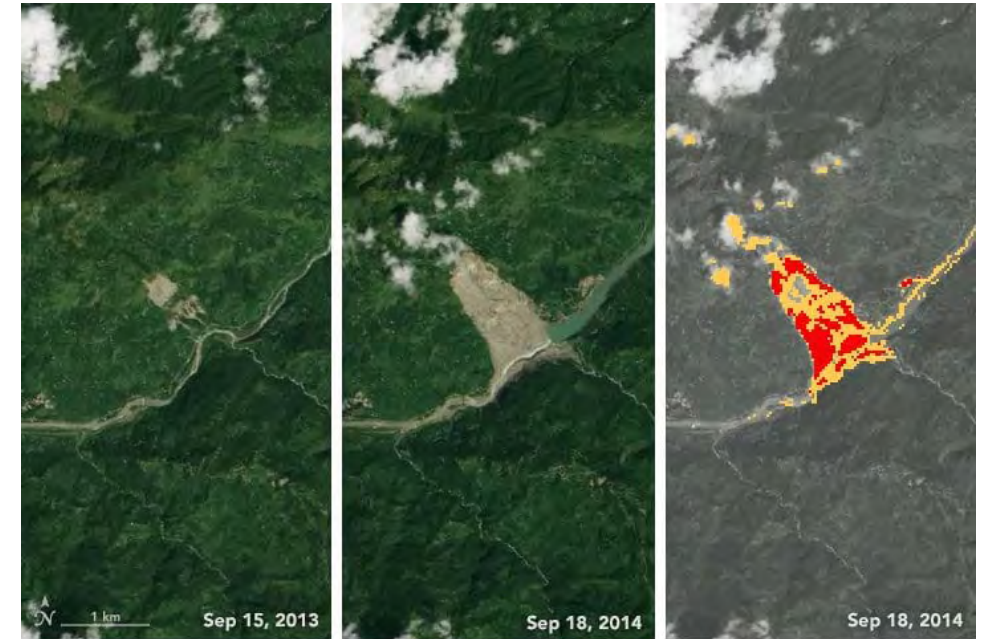
FIELD SURVEY



AERIAL PHOTOS

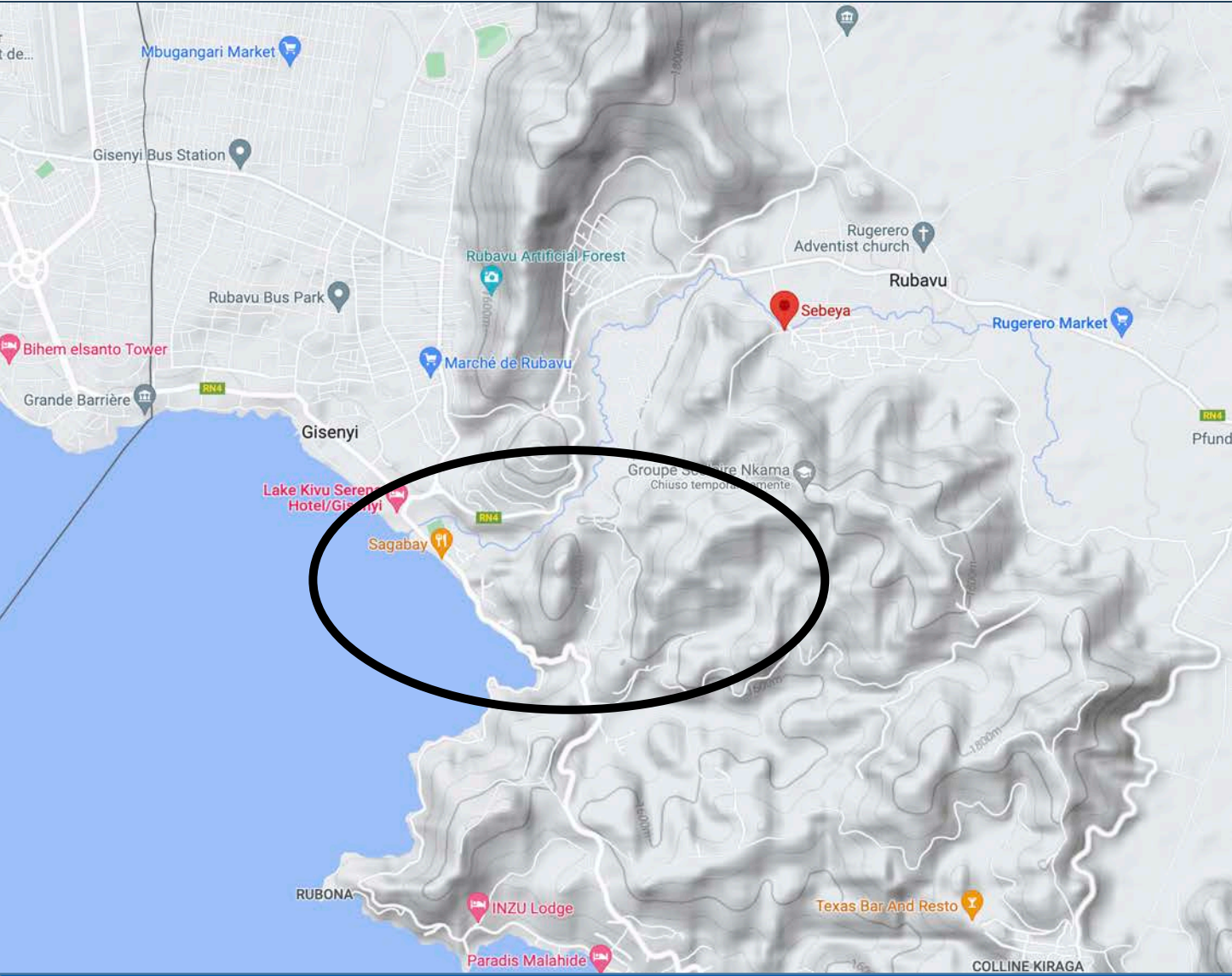


SATELLITE IMAGES



Rwanda rainfall event 2/3 May 2023: landslides and floods

Area



Sebeya Cathment

Landslides

Length: 280m

Area: 0.031 Km²

Pre-Event (10/04/2023)



Post-Event (20/05/2023)



Landslides

Pre-Event (10/04/2023)



Post-Event (20/05/2023)



Landslides

Pre-Event (10/04/23)

Post-Event (20/05/23)



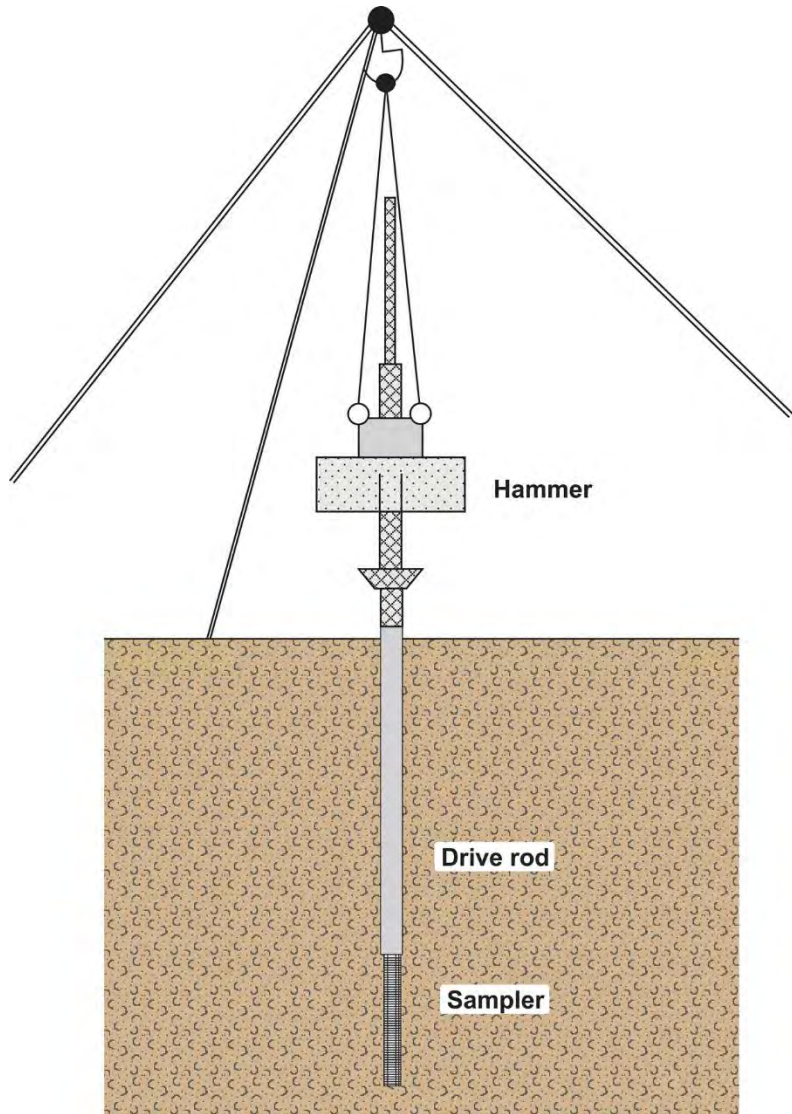
First goal: INVENTORY MAP

Second goal: DATA COLLECTION = SOIL CHARACTERIZATION

Soil samples for lab analyses



Geotechnical drilling and sampling



Geotechnical drilling and sampling

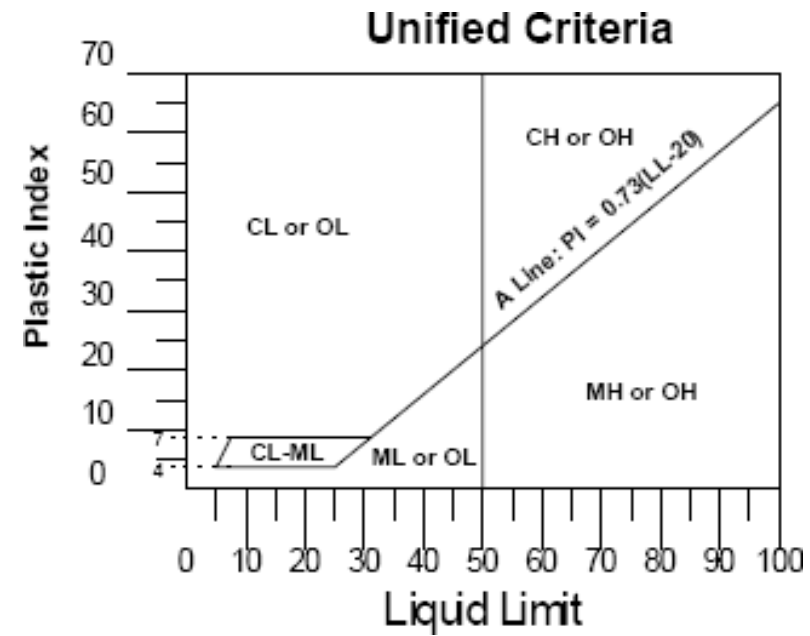
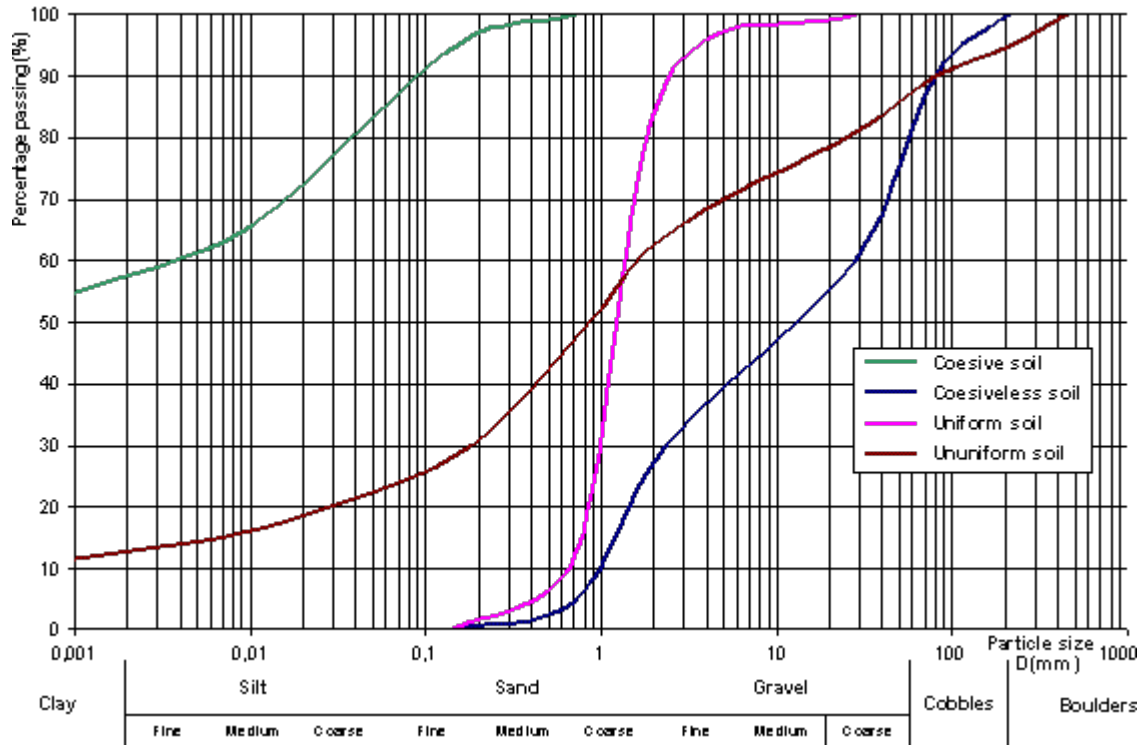
STRATIGRAPHY									
Riferimento: "Metodo per dimensionamento Minipali Geosystem"					Sondaggi: S1				
Località: fraz. Plerantorio - Comune di Umbertide					Quota:				
Impresa esecutrice: Edisystem srl					Data: 19 luglio 2021				
Perforazione: carotaggio continuo					Redattore: Dott. Geol. Michele Alemanno				
Coordinate:									
Riv.	A	m.	Litologia	camp.	Standard Penetration Test			Prof. m.	DESCRIPTION
					m.	S.F.T.	N.		
								0,5 m.	terreno di riporto antropico
1								1,2 m.	terreno vegetale
2								2,5 m.	Sabbie limose color nocciola con piccole concrezioni calcaree
3				C1					Sabbie fini e sabbie limose color nocciola con sfumature grigie
4					4,7	3/3/5	8	5,1 m.	
5									Limf e limf sabbiosi con orizzonti sabbiosi color nocciola con sfumature grigie e poca e presenza di frustoli vegetali
6									
7									
8									
9				C2				9,0 m.	
10					9,5	3/5/15	20	10,6 m.	Sabbie medie passanti a grossolane color nocciola marroni
11									Ghiale grossolane con clasti centimetrici subangolari e subarrotondati in matrice sabbiosa
12								12,2 m.	
13									Limf sabbiosi argillosi grigio-blauasti con frazione sabbiosa sabbiosa che aumenta con la profondità
14									
15				C3				15,0 m.	
16					15,45	nt.	nt.	15,5 m.	Ghiale grossolane con clasti centimetrici subangolari e subarrotondati in matrice sabbiosa



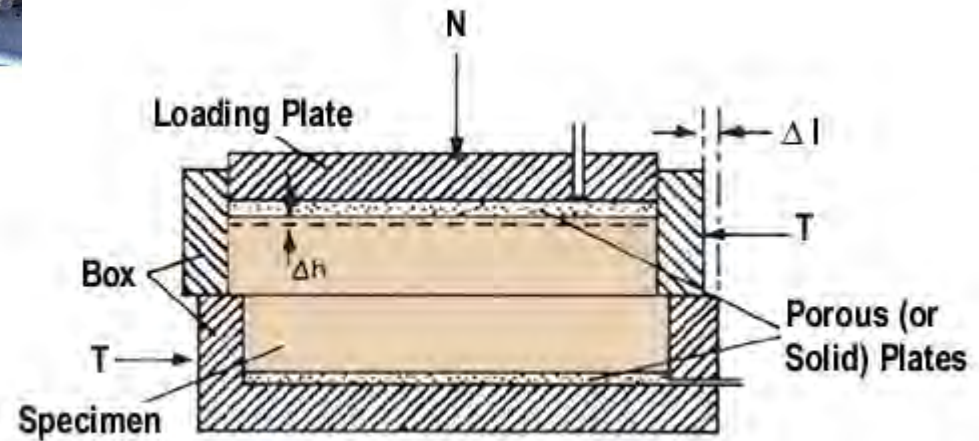
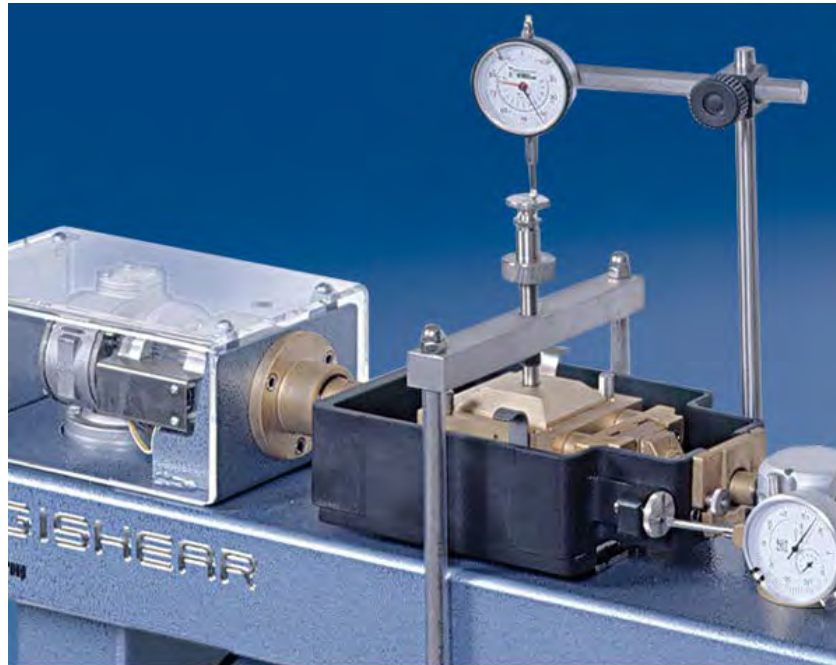
Soil classification in the lab



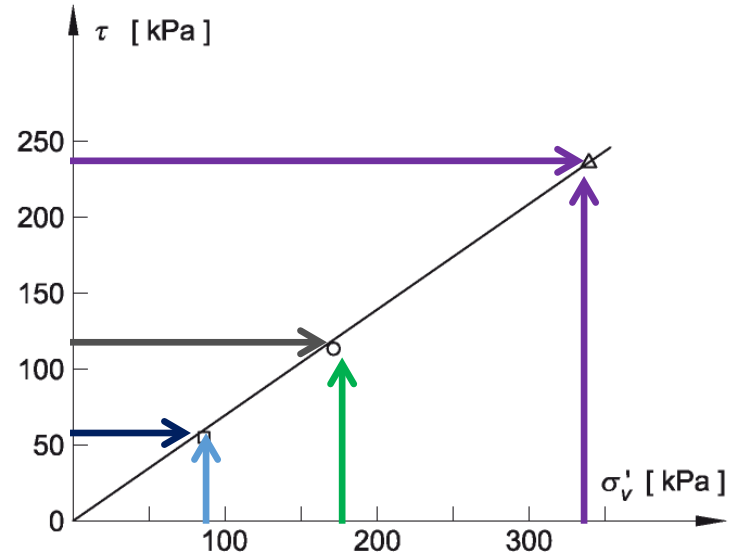
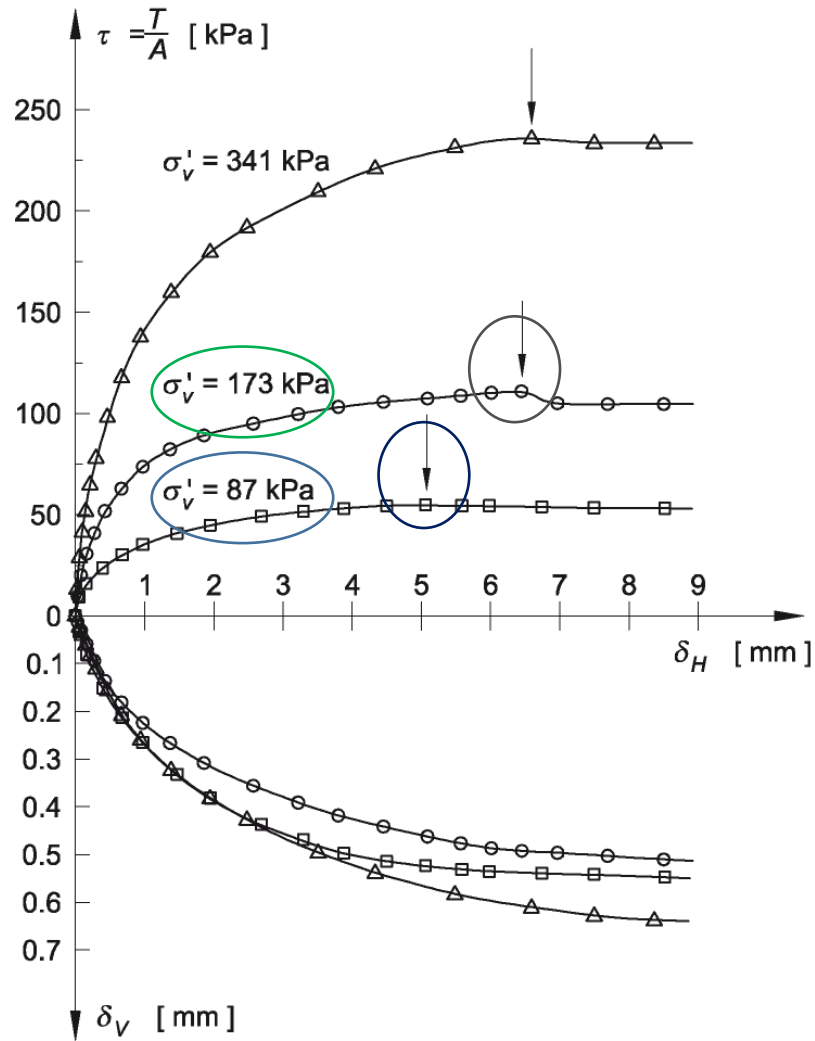
GRAIN SIZE DISTRIBUTION CURVE



The shear box for DIRECT SHEAR TEST



Direct shear test



MODENA CLAY

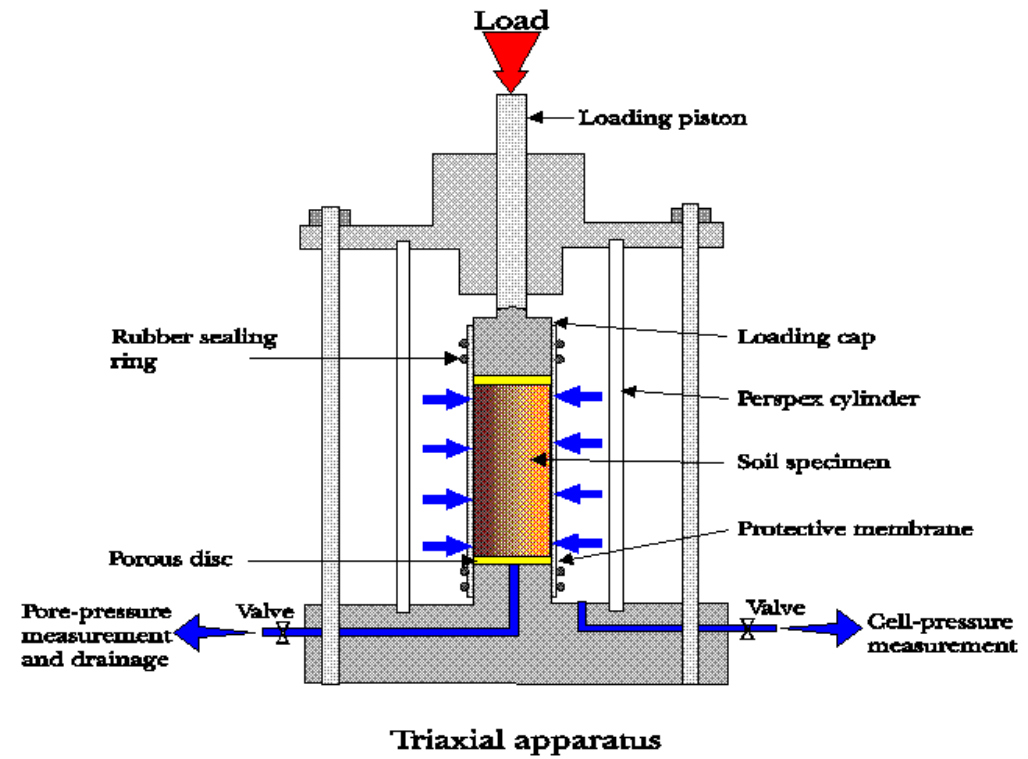
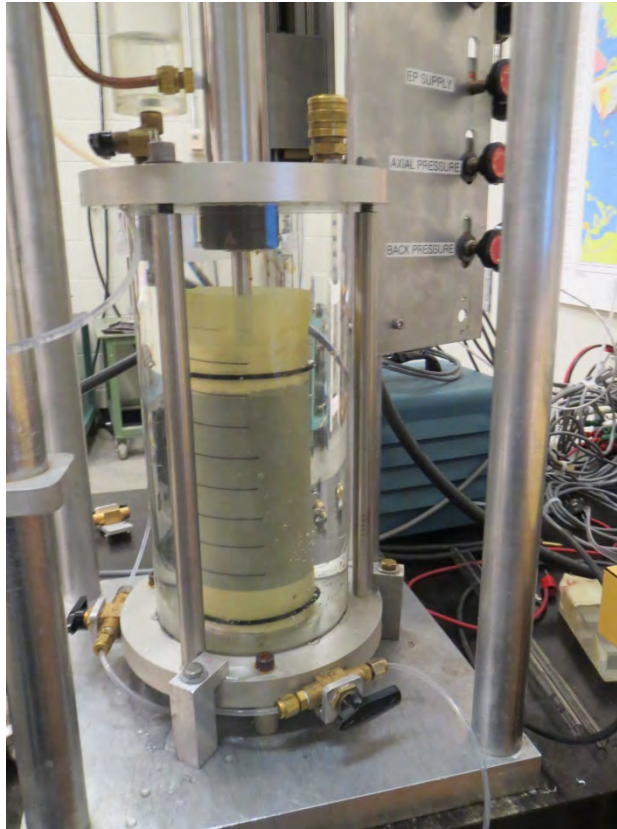
Prof. 8.36 m

$W_L = 36.3$ [%] $PI = 13.2$ [%]

$G_s = 2.739$ [-] $CF = 27.0$ [%]

$v = \frac{\delta_H}{t} = 0.006$ [mm/min]

Triaxial tests



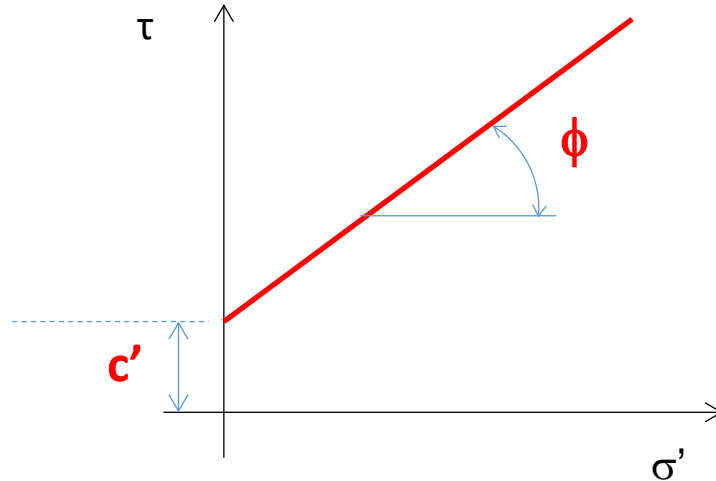
Mohr-Coulomb shear strength parameters

The shear strength of soils (and rocks) is expressed by two mechanical parameters coming from the Mohr-Coulomb criterion:

ϕ = friction angle

+

c' = effective cohesion



$$\tau_f = \tan\phi \cdot \sigma' + c'$$

ϕ and c' are also called **shear strength parameters of soils**

Landslide inventory (+ MAP):

- Date (day/month/year)
- Position (on the map)
- Shape (edge as polygon on the map)/2D profile
- Type
- Type of material (rock of soil)
- New or re-activated
- Triggering factors
- Land use
- Damage
- Photo

Data collection:

- ✓ Soil/rock samples
- ✓ Lab tests for classification and mechanical characterization

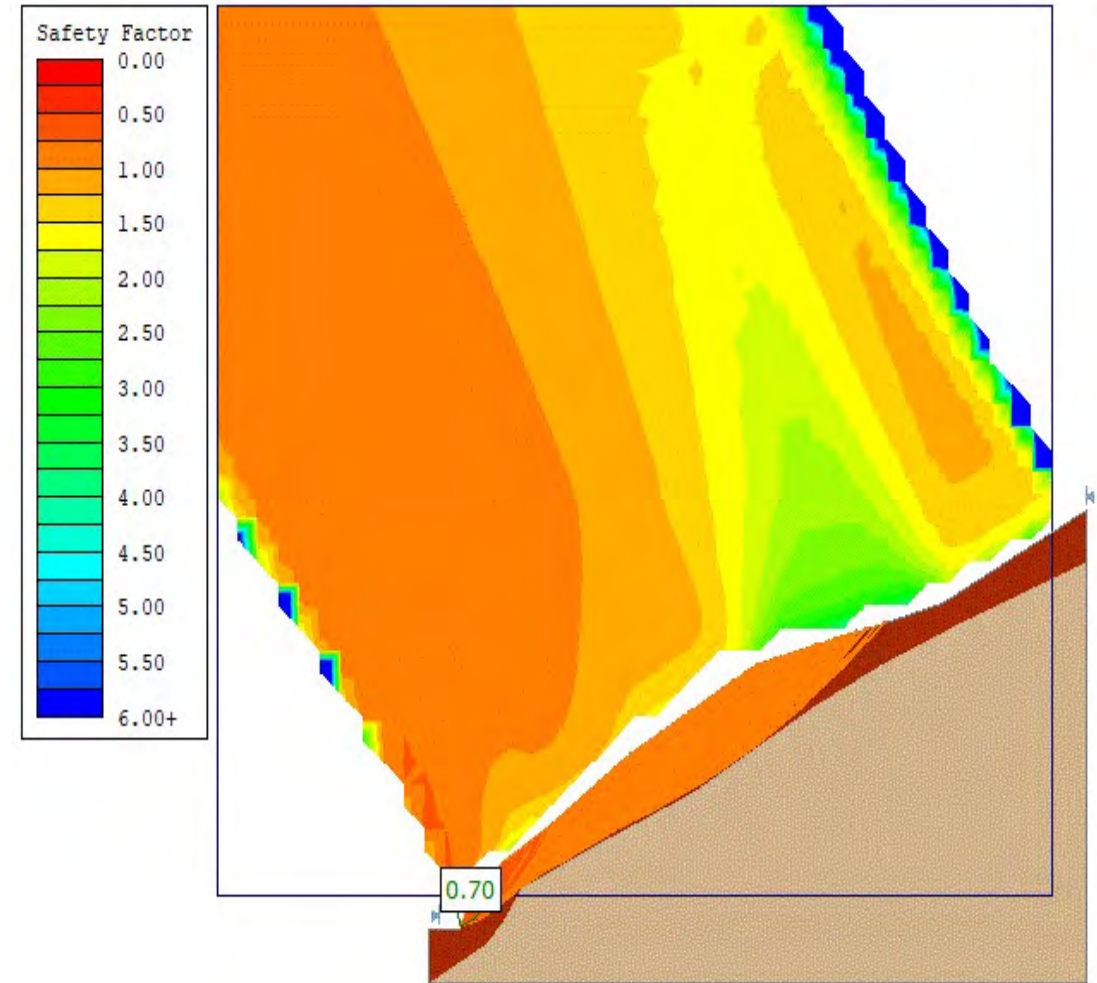
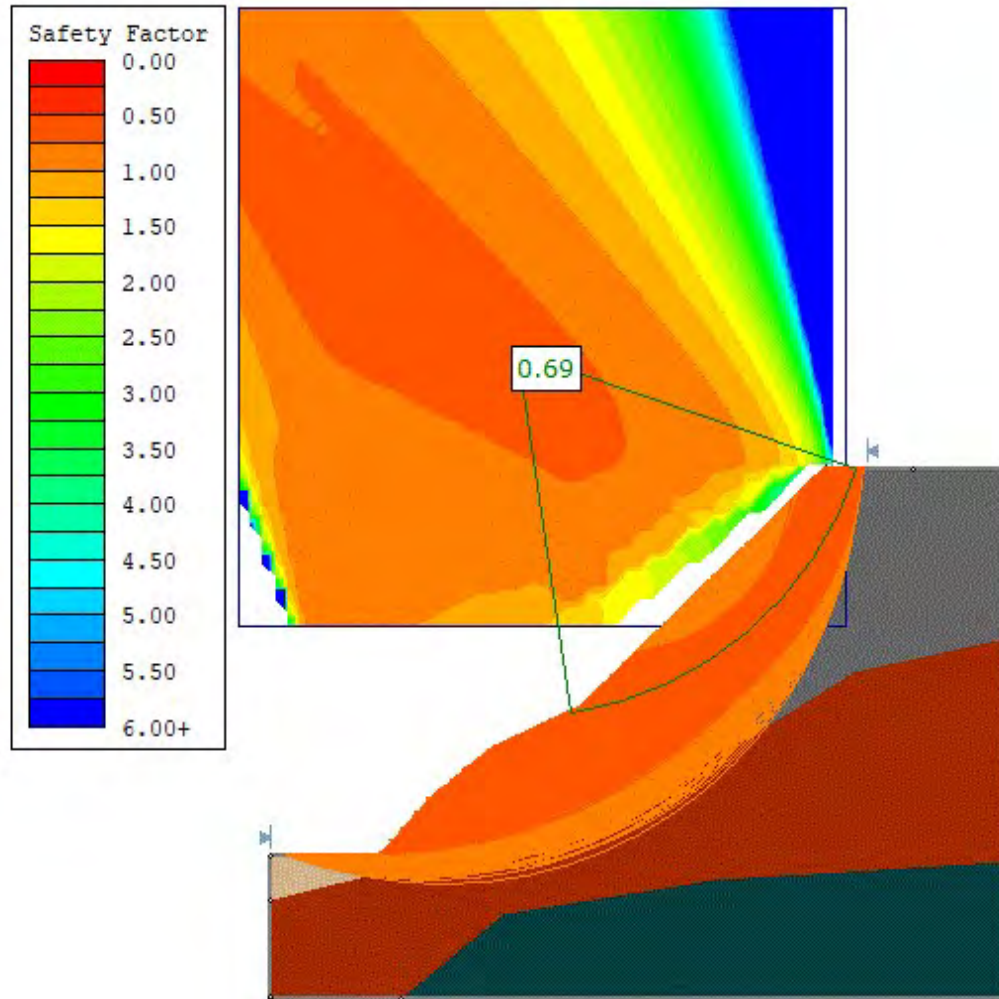
SINGLE SLOPE SCALE

**LARGE SCALE
(Village, District,
Catchment, Region,
Country)**

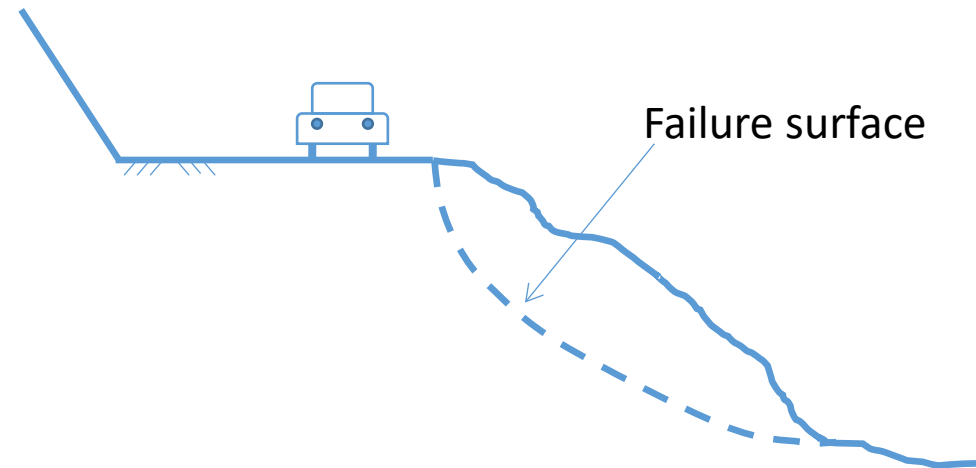
LEM

SLOPE ANALYSIS AT SINGLE SLOPE SCALE

SLOPE ANALYSIS AT SINGLE SLOPE SCALE



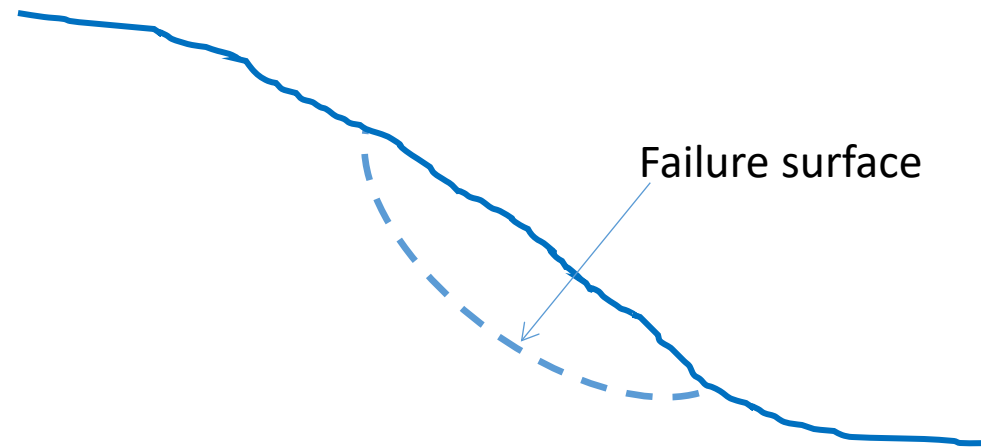
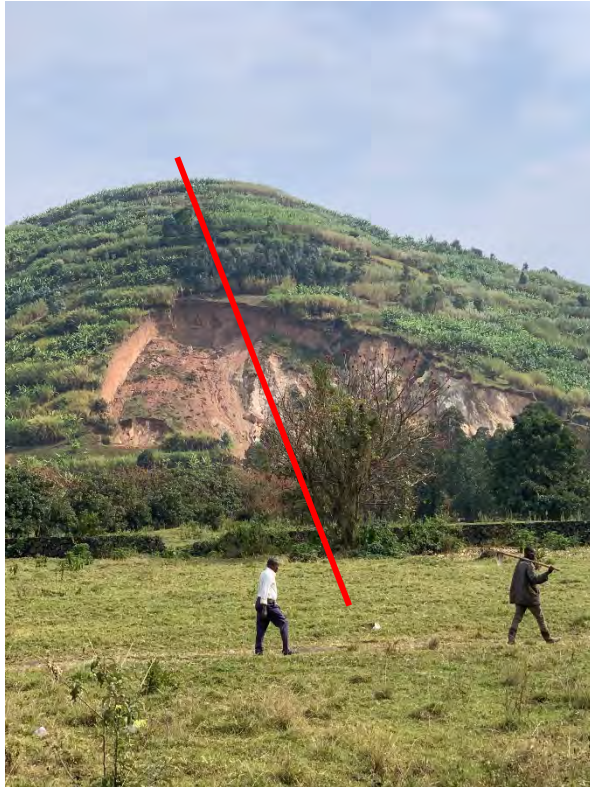
Slope stability: some examples



Natural or artificially constructed slopes may be stable at the moment, but they might lose their stability and cause collapse.

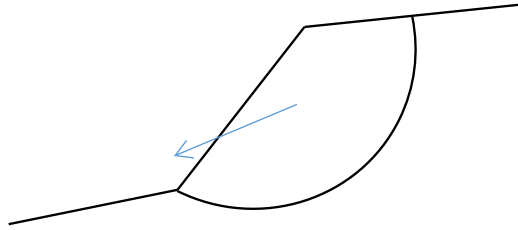
When it happens, the collapse can be sudden and catastrophic .

Slope stability: some examples

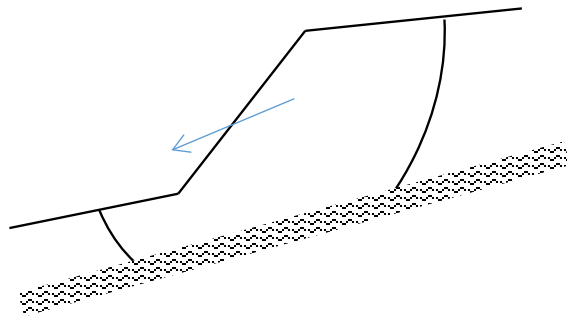


Slope stability

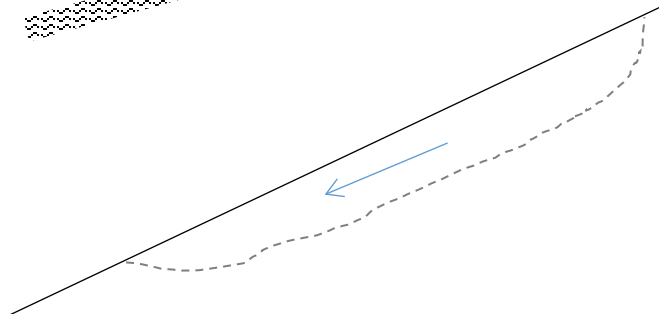
Several failure modes:



Rotational failure



Rotational-planar failure



Translational failure

Safety factor of a slope

Definition of «Safety factor» or «Factor of safety»

$$F_s = \frac{\text{Stabilizing actions}}{\text{Destabilizing actions}}$$

- > 1 **Stability**
- = 1 **Limit of equilibrium**
- < 1 **Instability**

Stabilizing actions



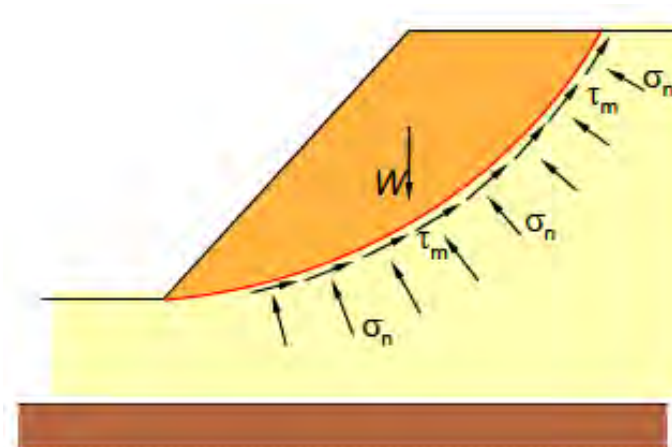
Soil shear strength



$$\tau_f = c' + (\sigma_n - u) \cdot \tan\phi'$$



$$F_s = \frac{\tau_f}{\tau_m}$$

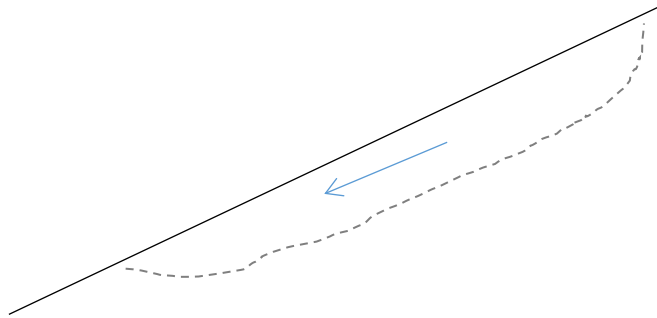


Limit Equilibrium Method

Assumptions

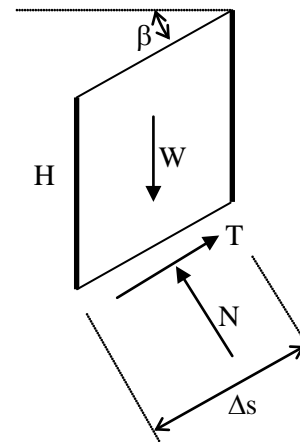
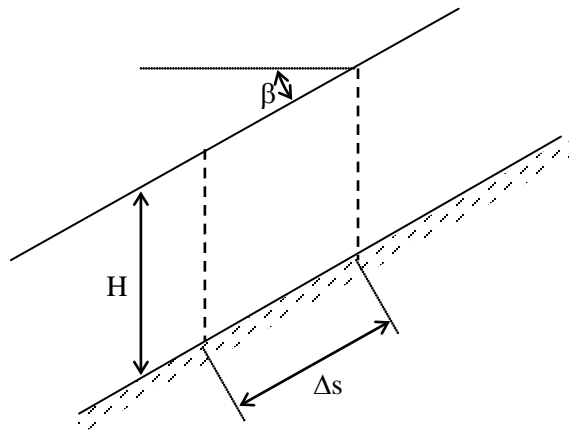
- The stress system is two-dimensional. Stresses in the third direction (perpendicular to the soil mass section) are assumed to be zero.
- The Mohr-Coulomb equation for shear strength is assumed to be valid, and the strength parameters ϕ' and c' are known in drained conditions (c_u is known in undrained conditions).
- Seepage conditions and water levels are known and pore water pressure can be calculated.
- Plastic failure conditions are assumed to be met along the critical/failure surface. In other words, shearing strains are large enough at all points of the critical surface to mobilize all available shear strength at the same time (rigid-plastic behavior).

Slope stability: infinite slope (dry soil)



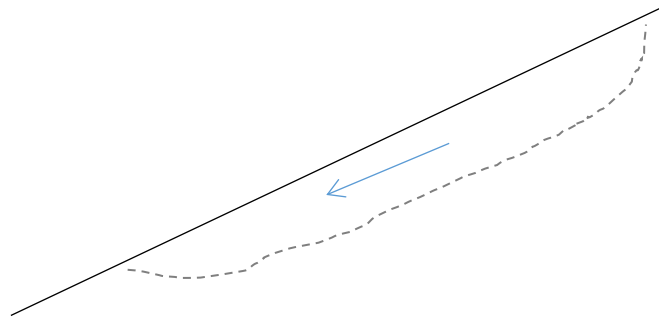
Translational failure

$\phi' \neq 0$ $c' = 0$ Dry soil



$$F_S = \frac{T_S}{T_D} = \frac{N \cdot \tan\phi'}{W \cdot \sin\beta} = \frac{W \cos\beta \cdot \tan\phi'}{W \cdot \sin\beta} = \frac{\tan\phi'}{\tan\beta}$$

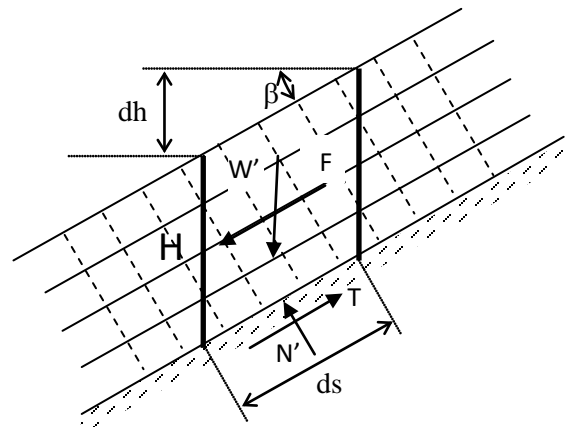
Slope stability: infinite slope (saturated soil with water table)



Translational failure

$$\phi' \neq 0 \quad c' = 0$$

water table at slope surface

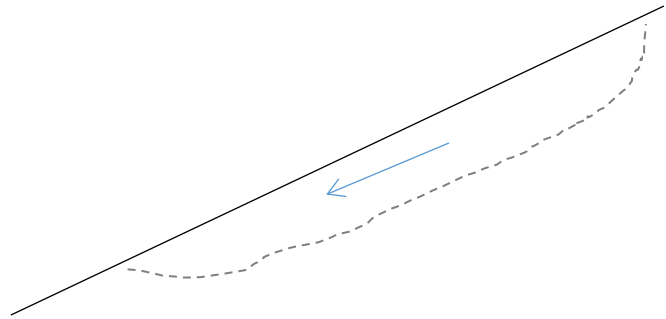


—— Water flow lines
 - - - Equipotential lines

$$F_S = \frac{T_S}{T_D} = \frac{N' \cdot \tan\phi'}{W' \cdot \sin\beta + F'} = \frac{\gamma' H \cdot ds \cdot \cos\beta \cdot \cos\beta \cdot \tan\phi'}{\gamma' H \cdot ds \cdot \cos\beta \cdot \sin\beta + \gamma_w H \cdot ds \cdot \cos\beta \cdot \sin\beta}$$

$$= \frac{\gamma'}{\gamma' + \gamma_w} \cdot \frac{\tan\phi'}{\tan\beta} = \frac{\gamma'}{\gamma_{sat}} \cdot \frac{\tan\phi'}{\tan\beta}$$

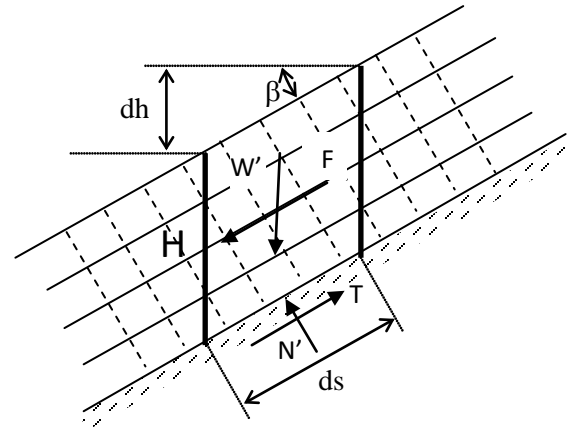
Slope stability: infinite slope (saturated soil with water table)



Translational failure

$$\phi' \neq 0 \quad c' \neq 0$$

water table at slope surface



- Water flow lines
- Equipotential lines

$$F_S = \frac{T_S}{T_D} = \frac{N' \cdot \tan\phi' + c' \cdot ds}{W' \cdot \sin\beta + F'} = \frac{\gamma' H \cdot ds \cdot \cos\beta \cdot \cos\beta \cdot \tan\phi' + c' \cdot ds}{\gamma' H \cdot ds \cdot \cos\beta \cdot \sin\beta + \gamma_w H \cdot ds \cdot \cos\beta \cdot \sin\beta}$$

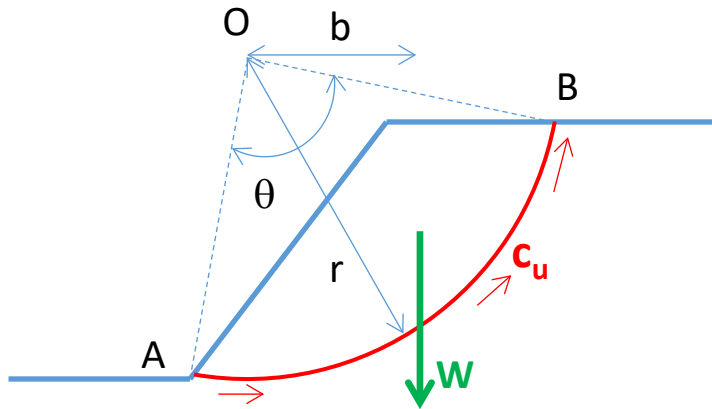
$$= \frac{\gamma' H \cdot \cos^2\beta \cdot \tan\phi' + c'}{\gamma_{sat} H \cdot \cos\beta \cdot \sin\beta}$$

Slope stability: circular slip surface in undrained conditions

Rotational failure

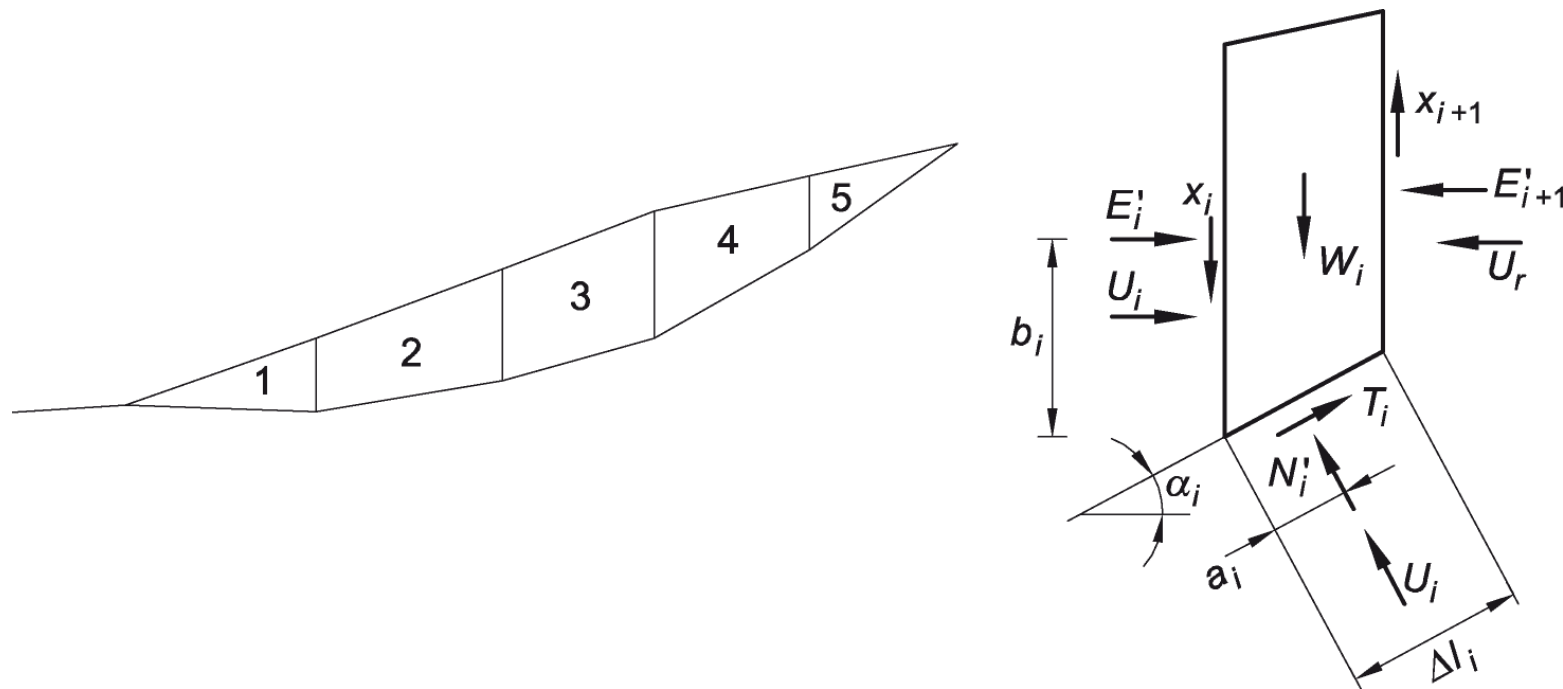
$$c_u \neq 0$$

Total stresses



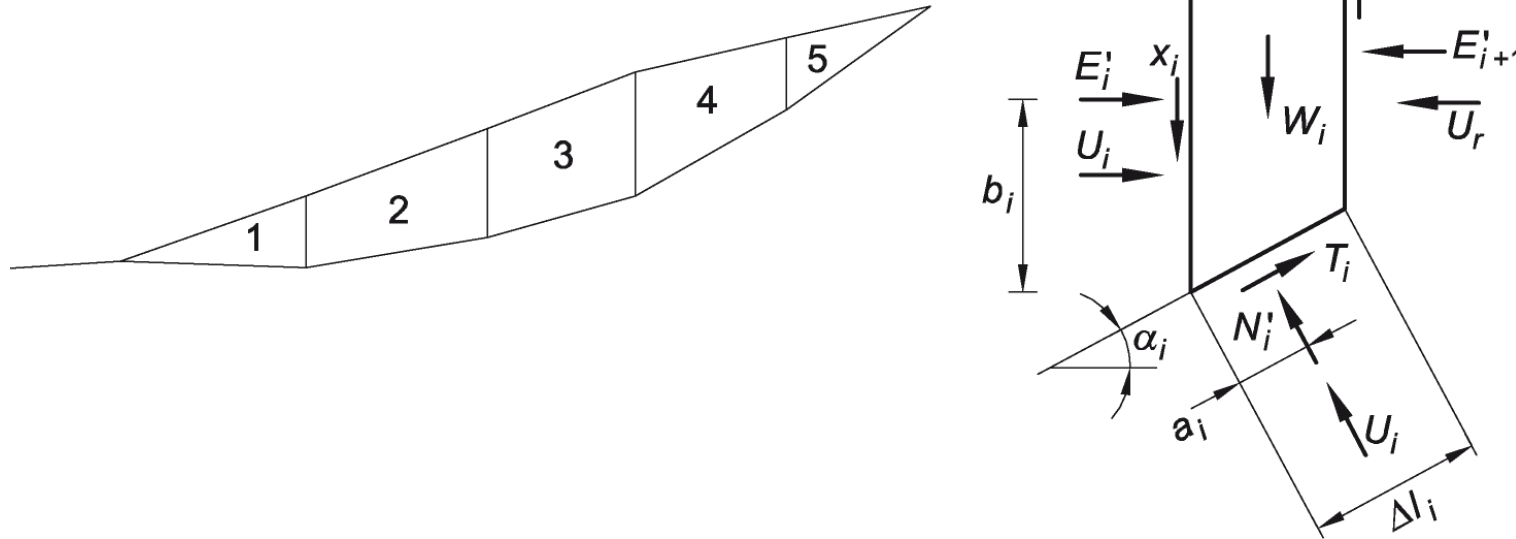
$$F_S = \frac{M_S}{M_D} = \frac{c_u \cdot r \cdot l_{AB}}{W \cdot b} = \frac{c_u \cdot r \cdot r \cdot \frac{\vartheta \cdot \pi}{180^\circ}}{W \cdot b}$$

Slope stability: slice method



- The potentially unstable soil mass is divided into ***n* slices**;
- ***Slices*** interact with each other and are delimited by the potential failure surface at the bottom;
- The discretization makes it possible to treat each individual ***slice*** as a single, potentially unstable “rigid” block;
- Each ***slice*** is subject to external forces and also forces (x_i ; E_i) acting at the interface between two slices.

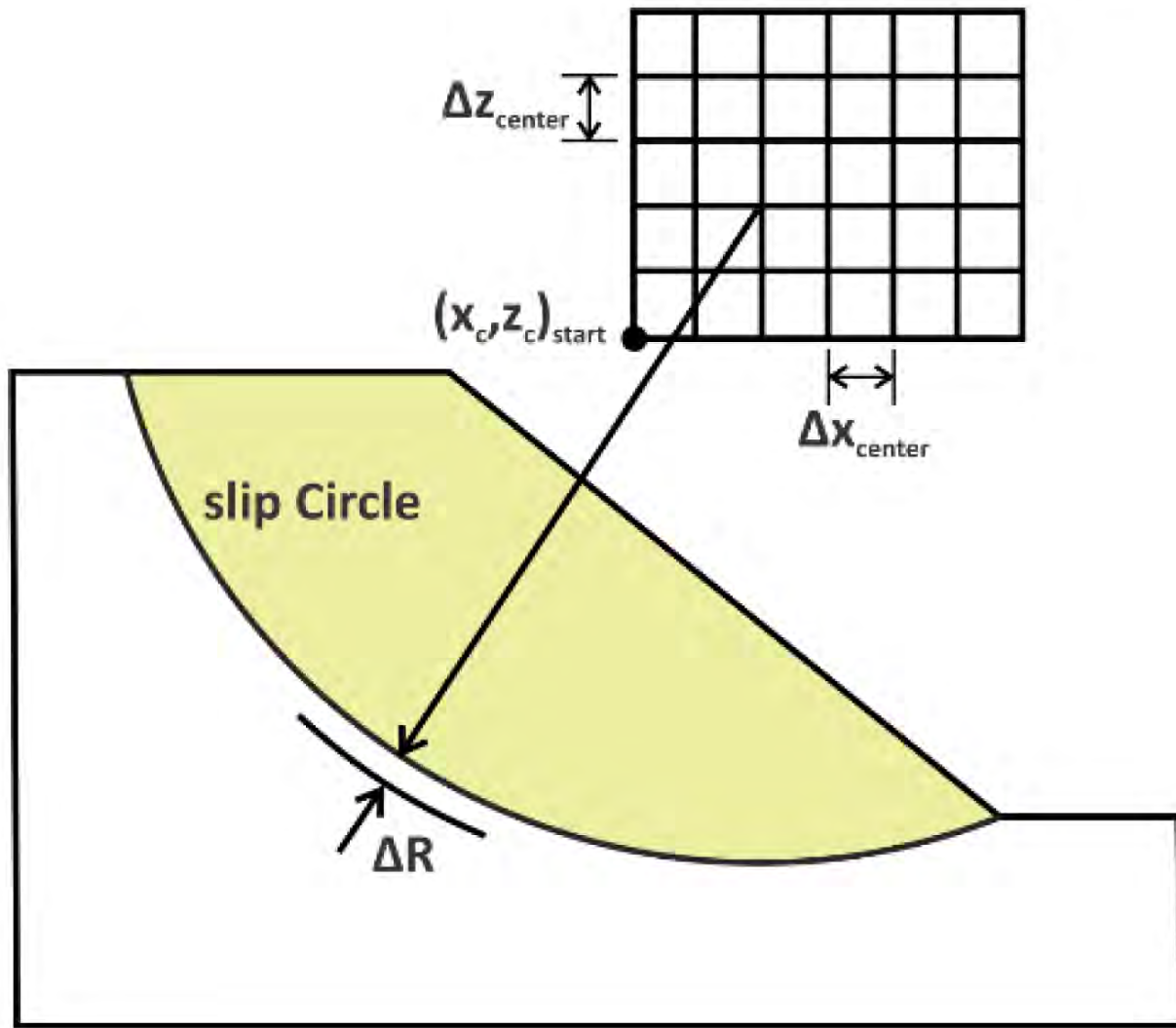
Slope stability: slice method



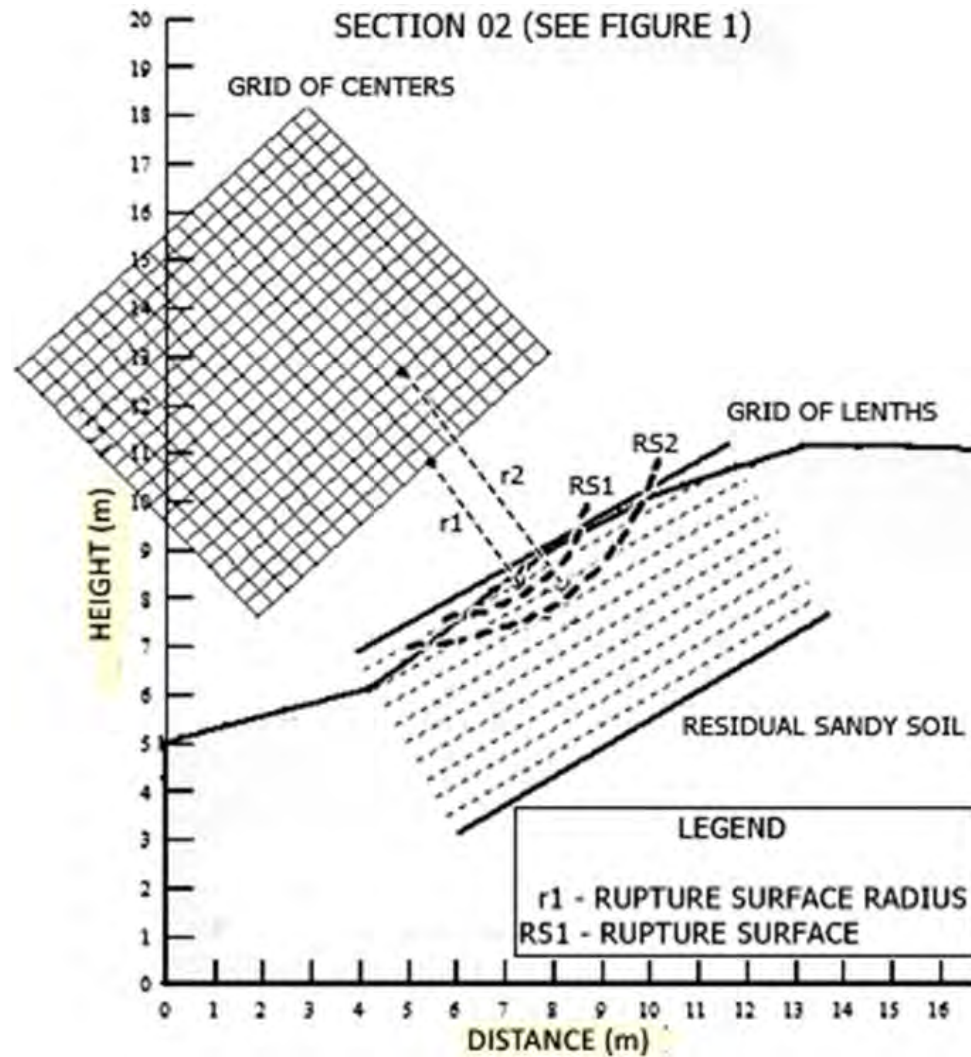
In addition to the interaction forces between slices, it is possible to take into account:

- different stratigraphic conditions in the subsurface;
- different ϕ' and c' values along the failure surface (as they relate to different segments);
- pore water pressures;
- variously distributed external forces;
- failure surfaces of any shape.

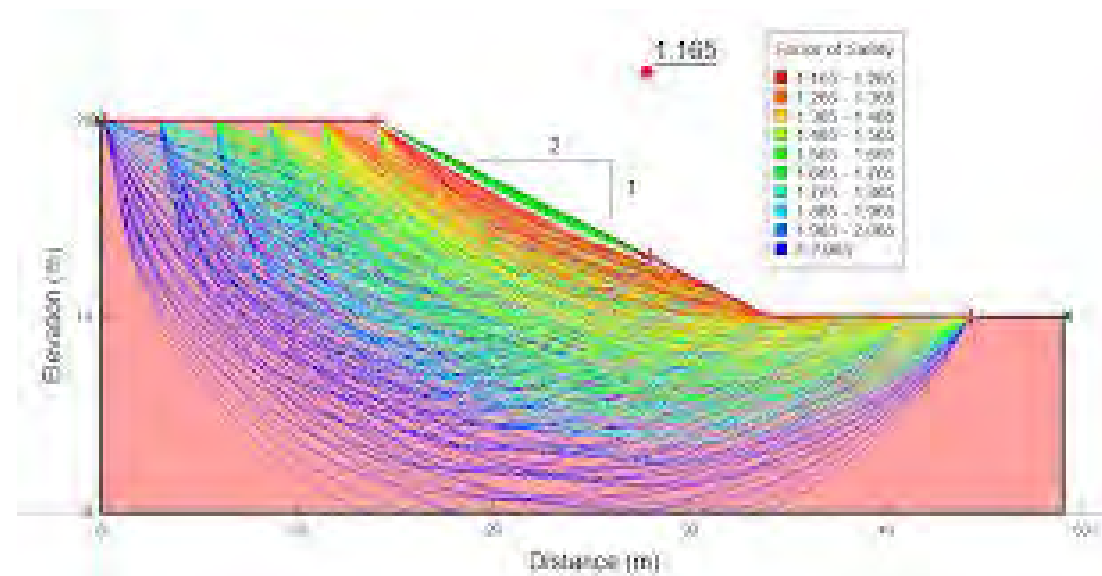
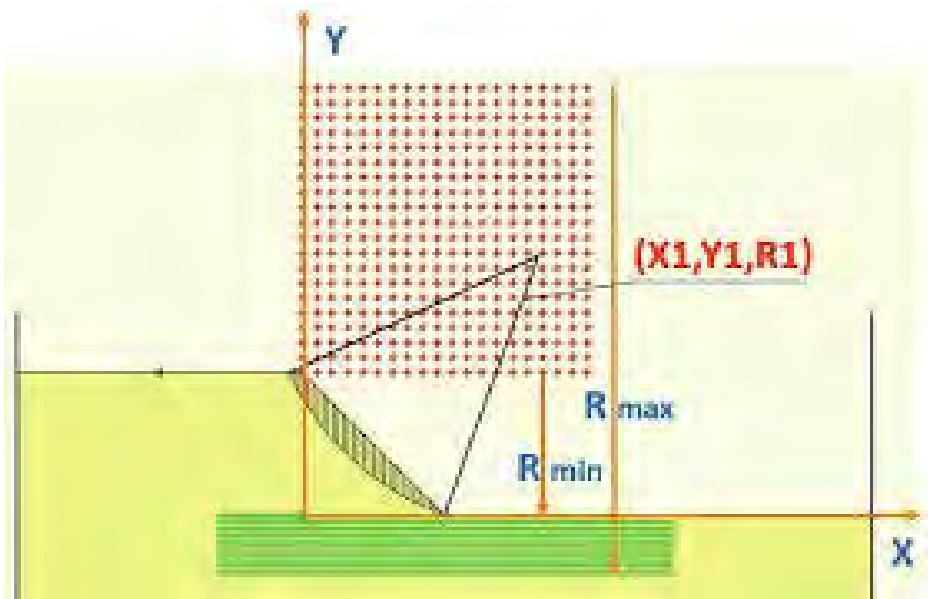
Slope stability: slice method



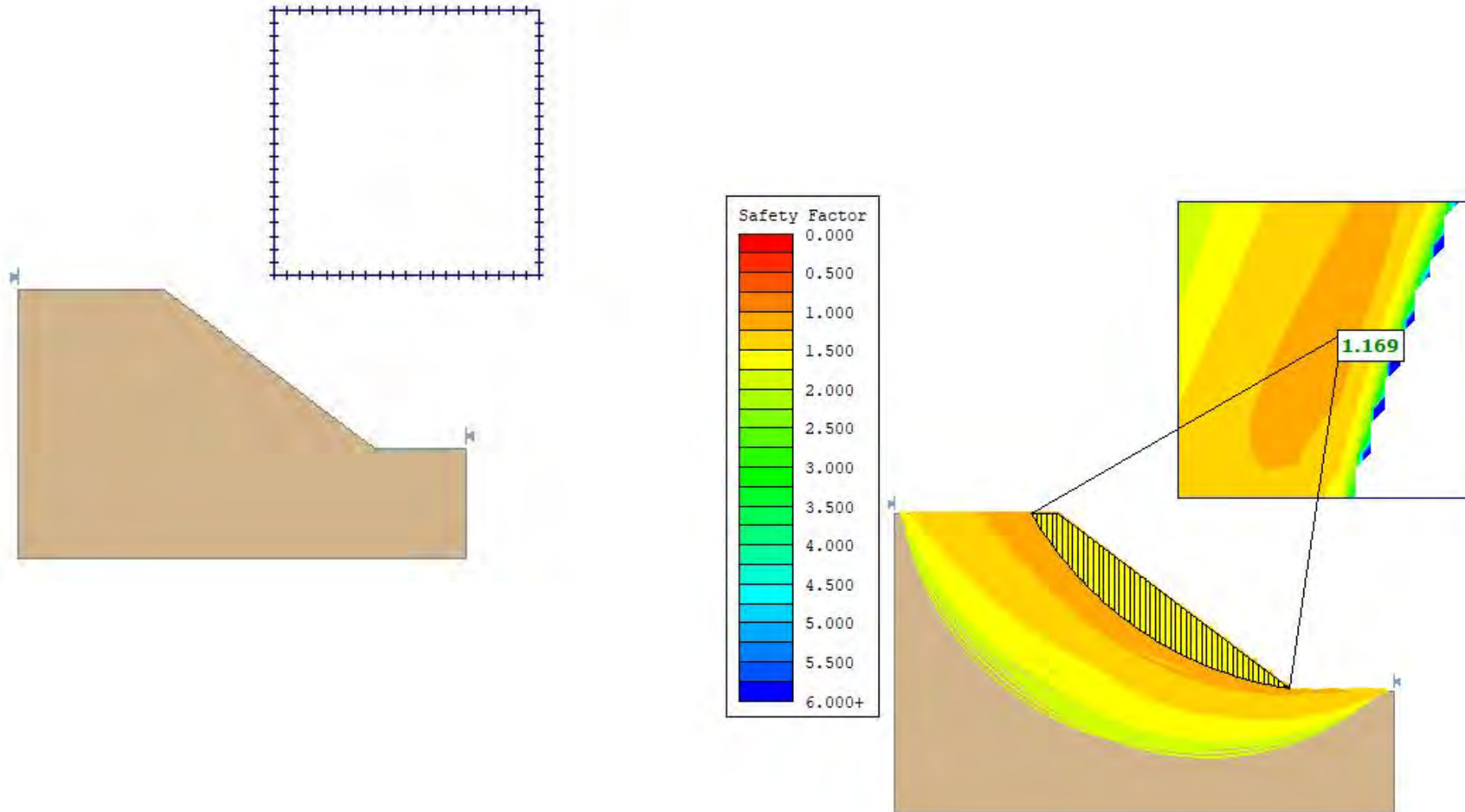
Slope stability: slice method



Slope stability: slice method



Slope stability: slice method

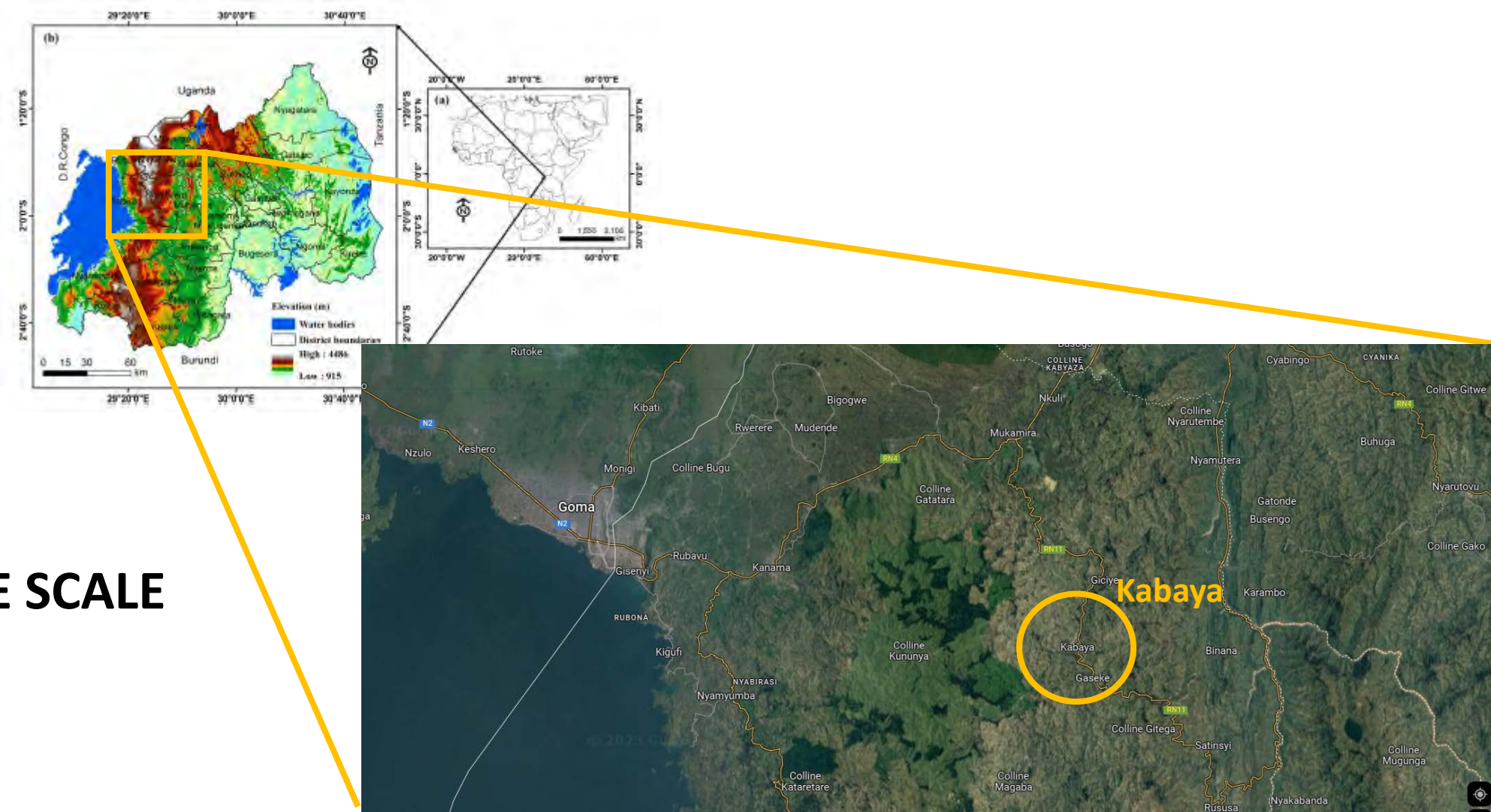


Free software



HYRCAN 2.0
©2022 Roozbeh Geraili Mikola

Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018



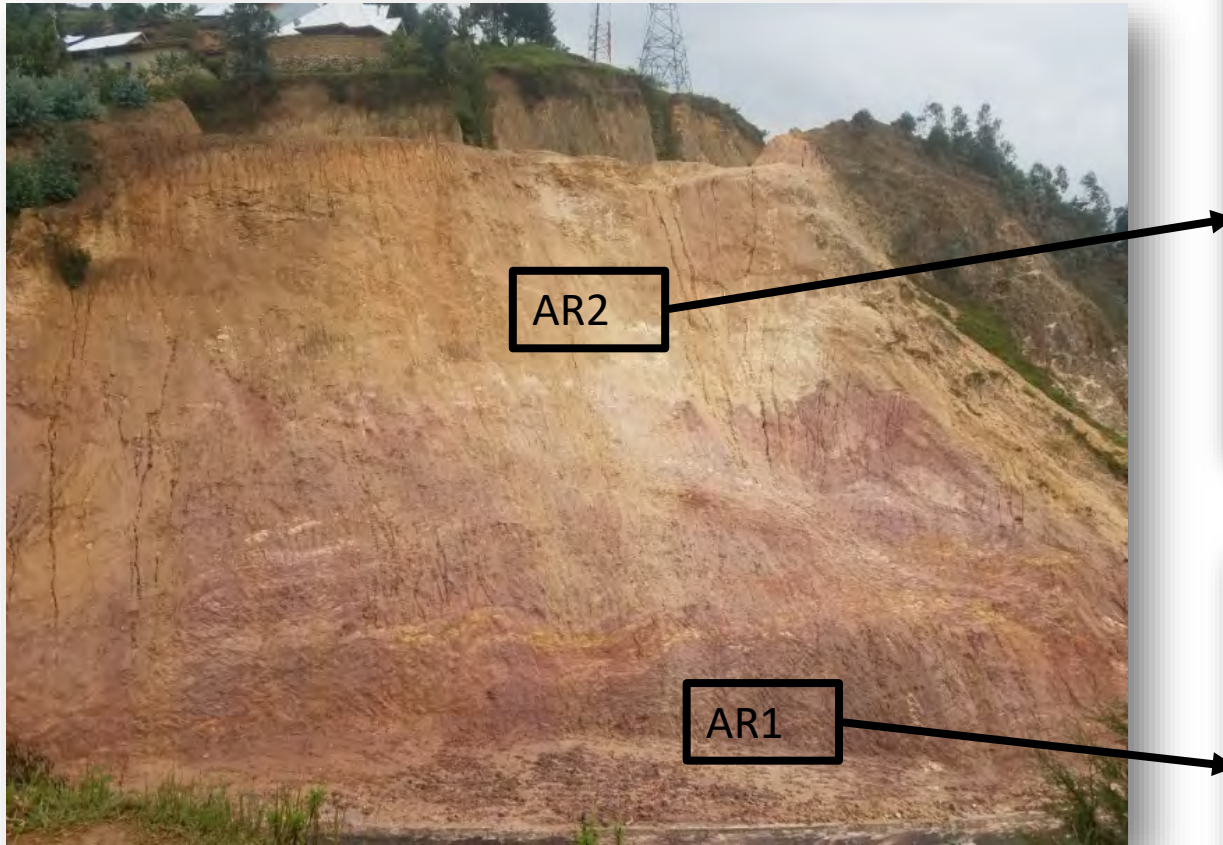
SLOPE SCALE

Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018



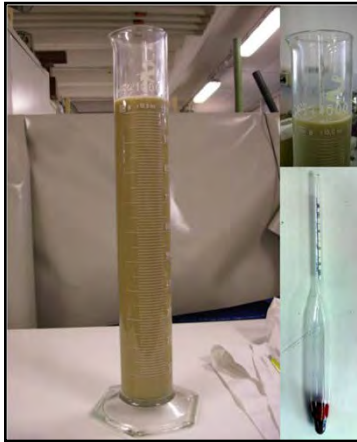
Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Collection of 2 soil samples



Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

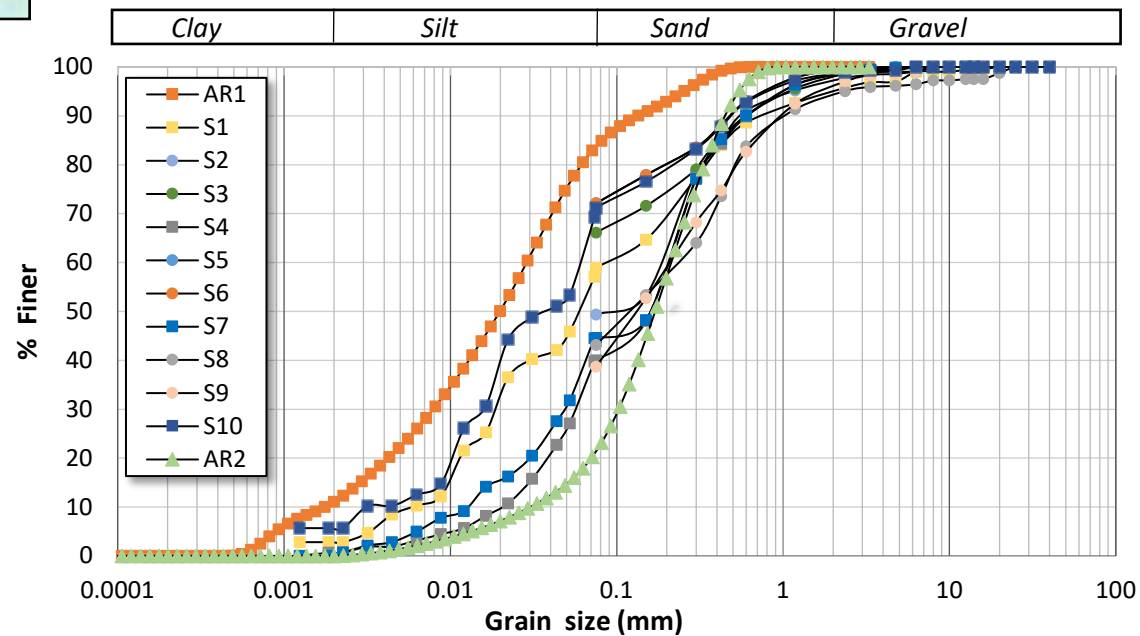
Sedimentation



Sieving

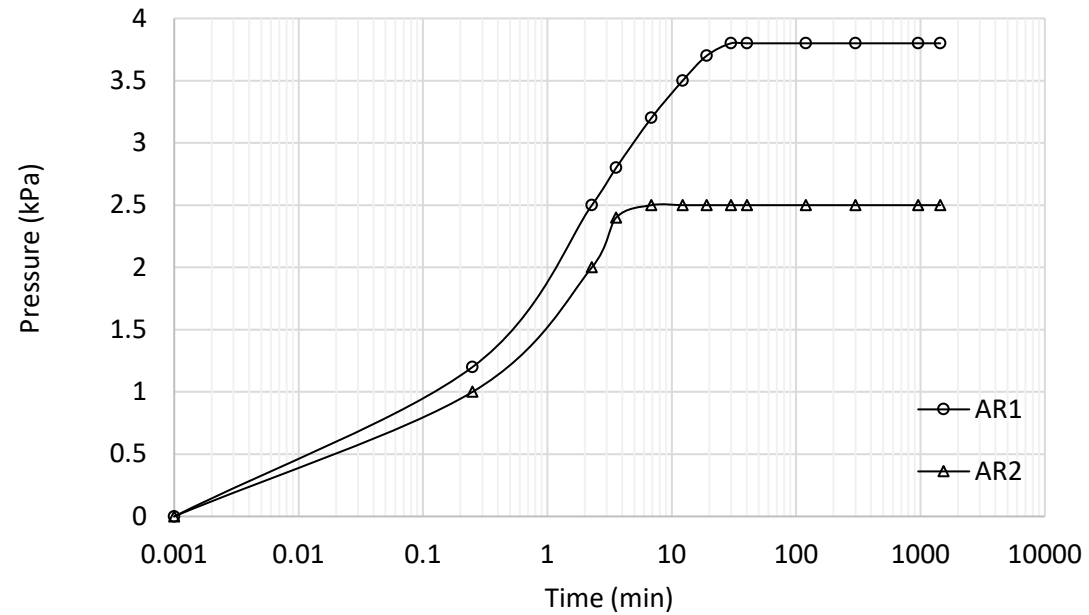


Laser optical granulometer



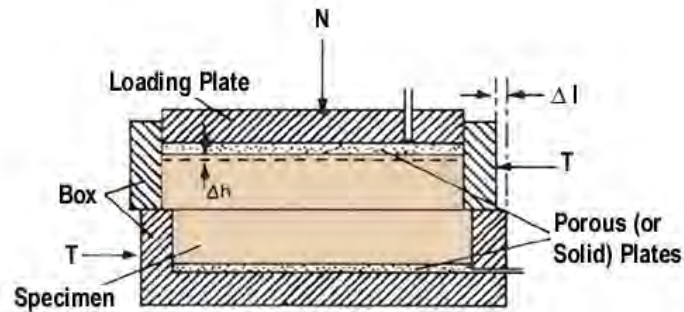
Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Swelling tests to determine the swelling pressure, by using the oedometer and the constant volume method (Method C) based on ASTM D4546-21

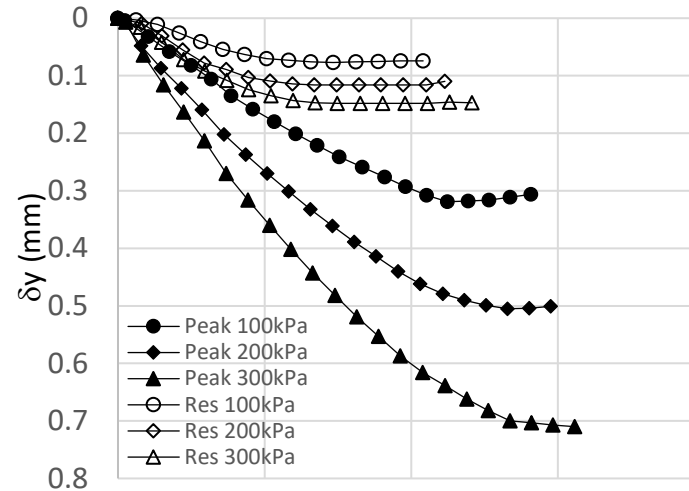
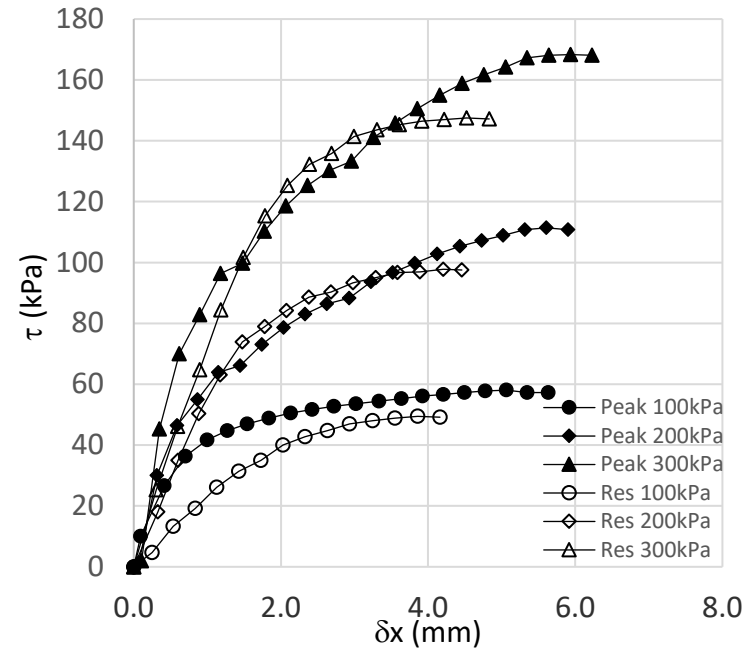


Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Direct shear test: mechanical characterization of soil (AR1)

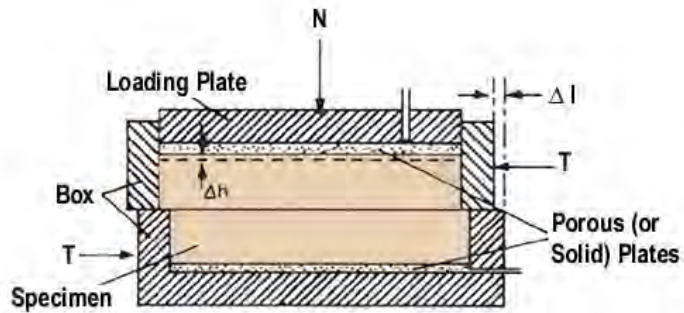


$$\tau_f = \tan\phi \cdot \sigma' + c'$$

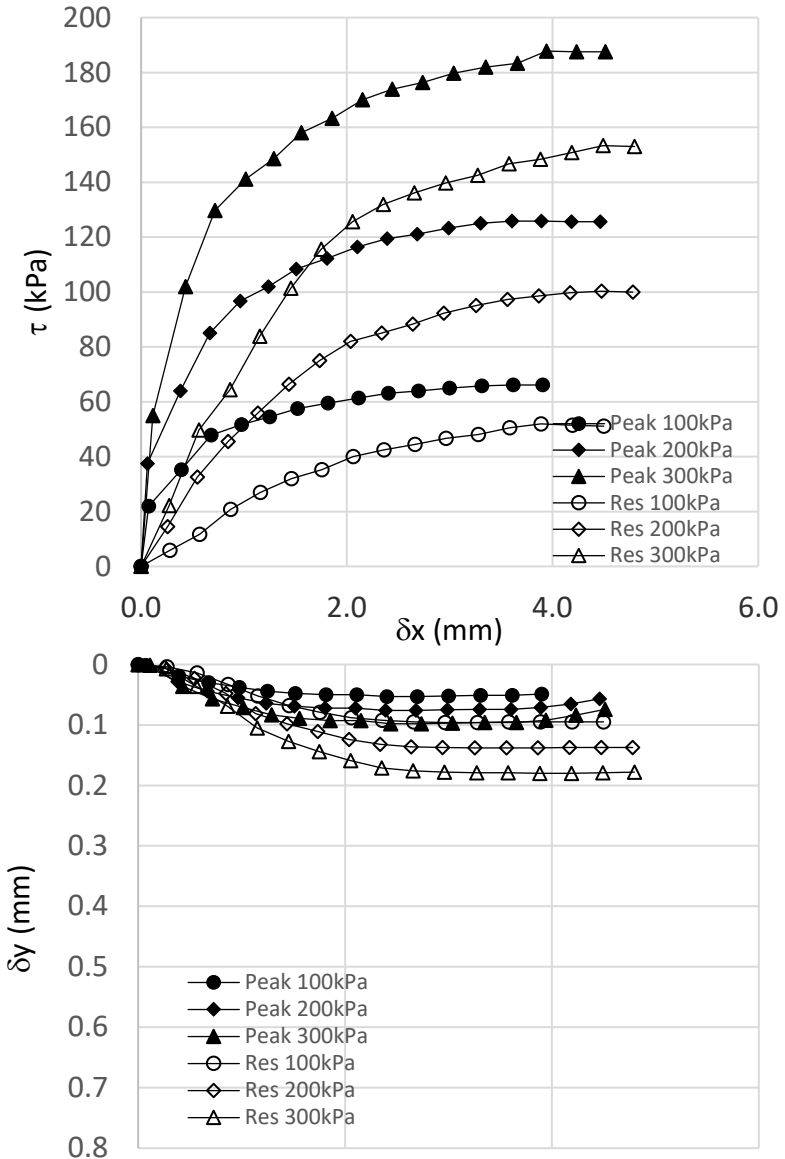


Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Direct shear test: mechanical characterization of soil (AR2)

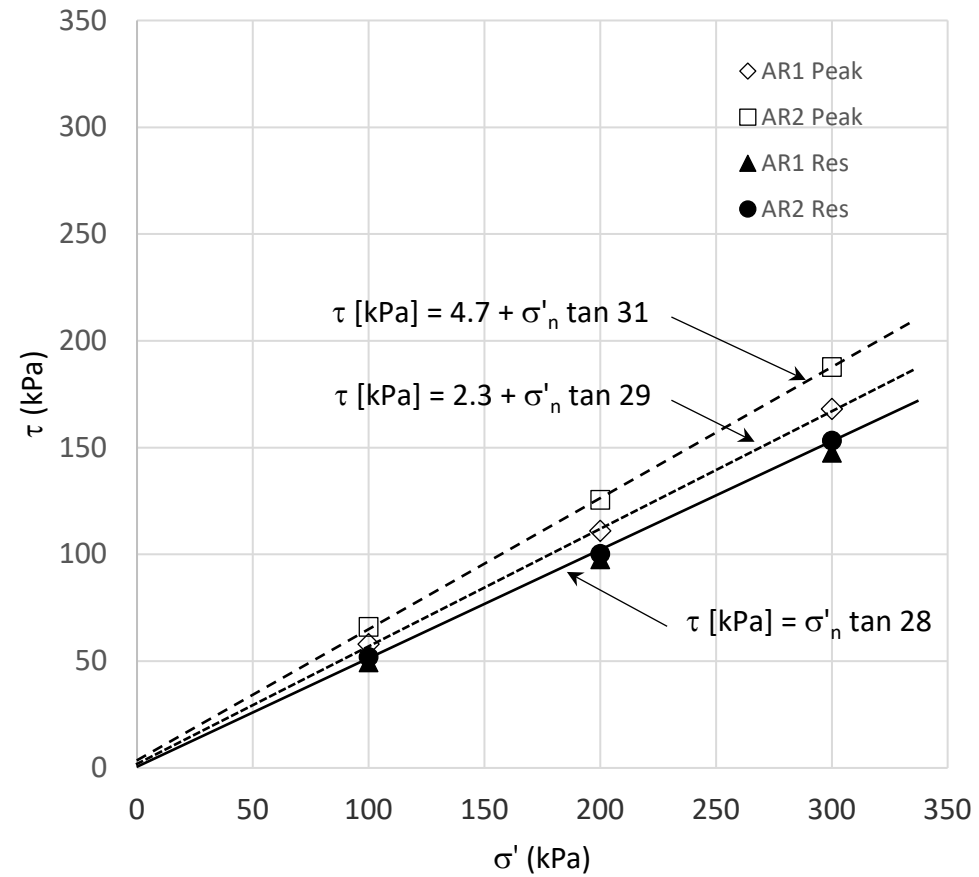


$$\tau_f = \tan\phi \cdot \sigma' + c'$$



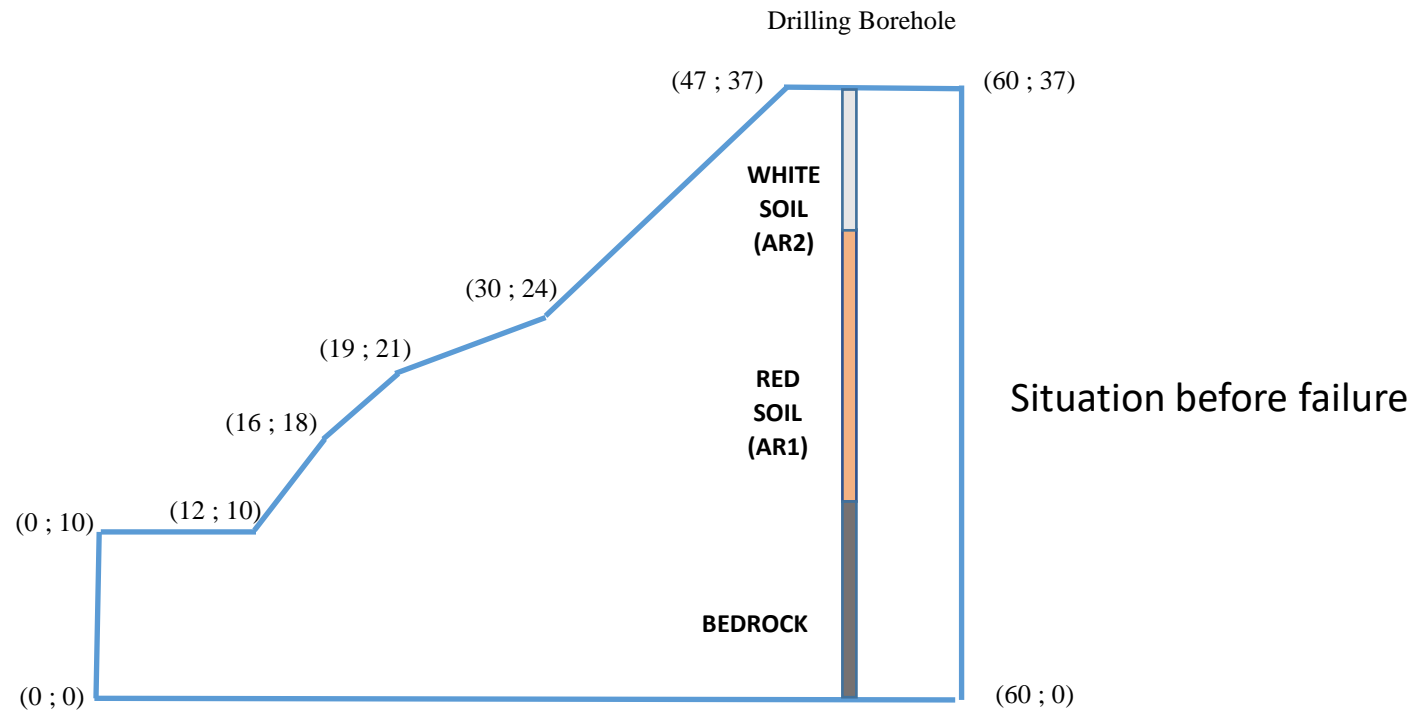
Mohr-Coulomb shear strength criterion

$$\tau_f = \tan\phi \cdot \sigma' + c'$$



Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Material	Top (m)	Bottom (m)	Unit weight (kN/m ³)	Effective cohesion c' (kPa)	Shear strength angle ϕ' (degree)
"White" soil (AR2)	0	8	17	4.7	31
"Red" soil (AR1)	8	26	18	2.3	29
Bedrock	26	37	23	50	35



Profile coordinates

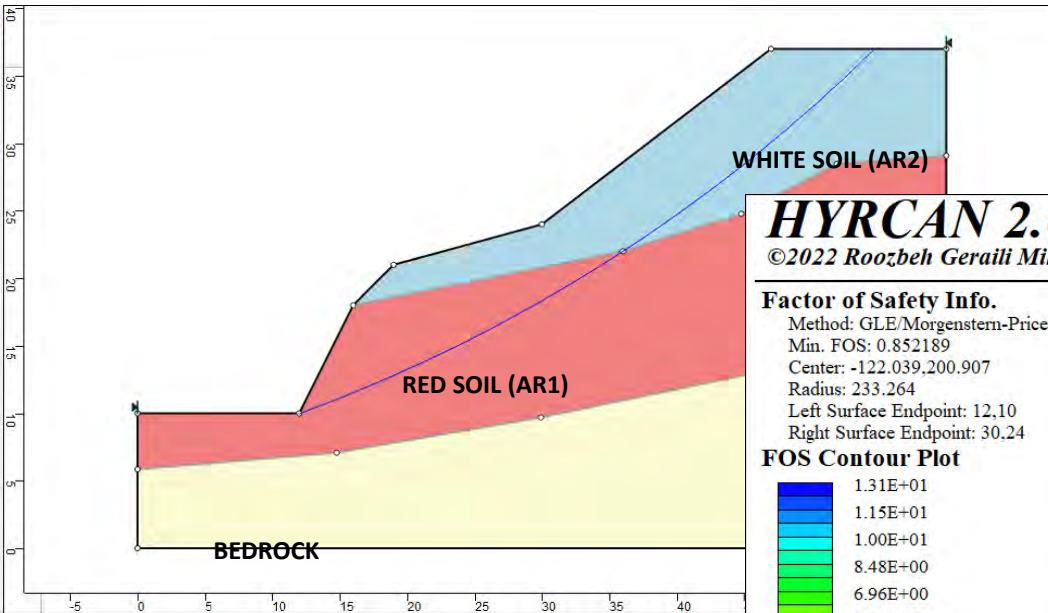
x	y
	0
0	10
12	10
16	18
19	21
30	24
47	37
60	37
60	0
0	0

Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

Material	Top (m)	Bottom (m)	Unit weight (kN/m ³)	Effective cohesion c' (kPa)	Shear strength angle ϕ' (degree)
“White” soil (AR2)	0	8	17	4.7	31
“Red” soil (AR1)	8	26	18	2.3	29
Bedrock	26	37	23	50	35

HYRCAN 2.0
©2022 Roozbeh Geraili Miko

Setting Information
Physical Unit: Metric
Failure Direction: Right to Left
Methods: Bishop Simplified, GLE/Morgenstern-Price, Janbu Simplified, Spencer
Assigned Soil Material:
AR2 (MC), AR1 (MC), Bedrock (MC)

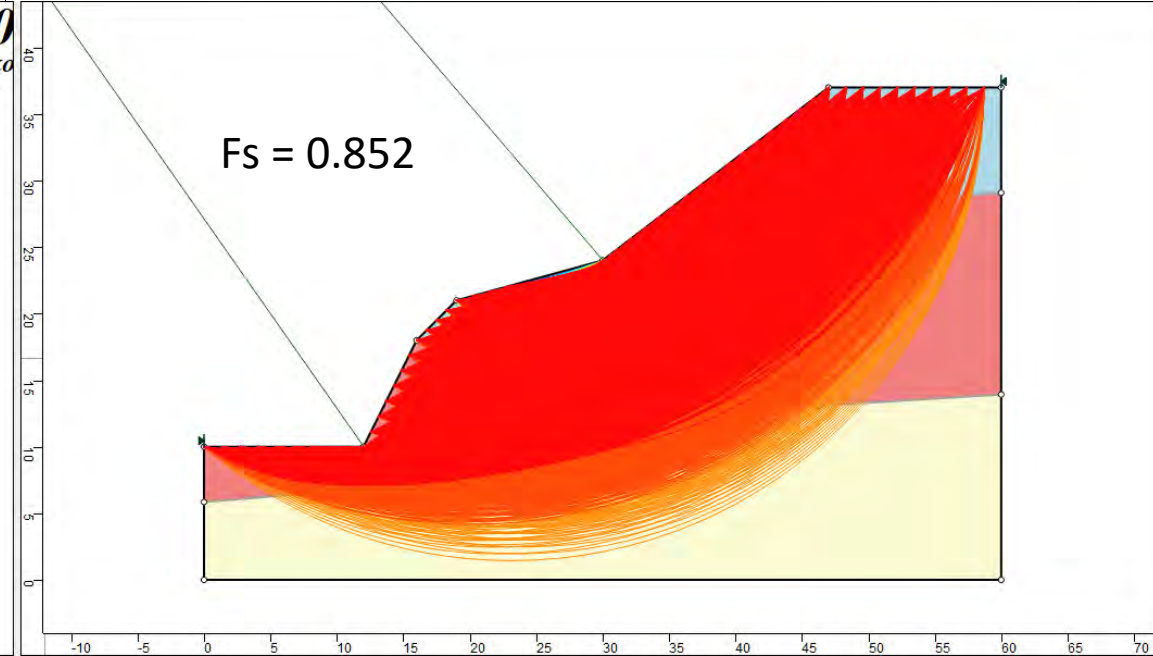


HYRCAN 2.0
©2022 Roozbeh Geraili Miko

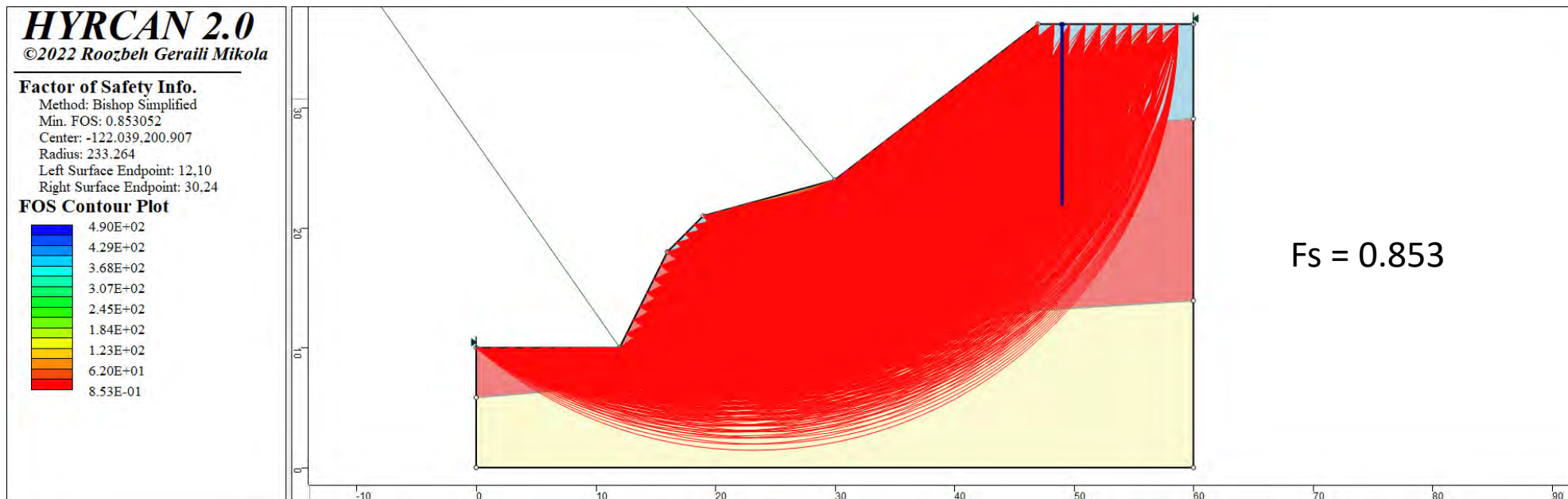
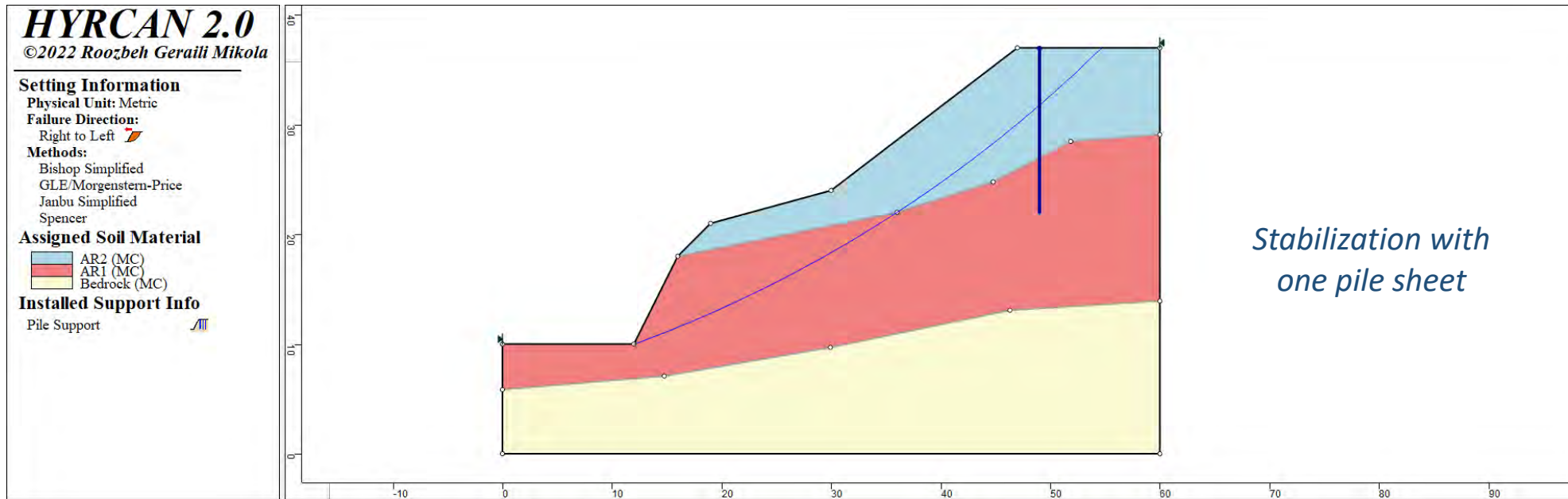
Factor of Safety Info.
Method: GLE/Morgenstern-Price
Min. FOS: 0.852189
Center: -122.039,200.907
Radius: 233.264
Left Surface Endpoint: 12,10
Right Surface Endpoint: 30,24

FOS Contour Plot

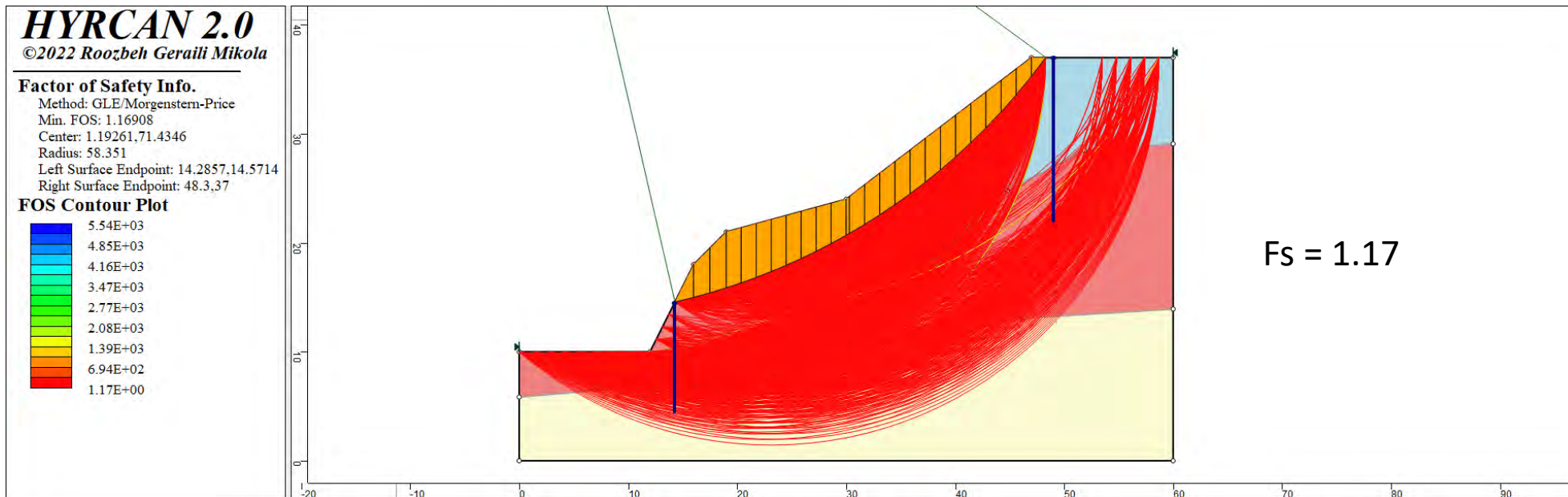
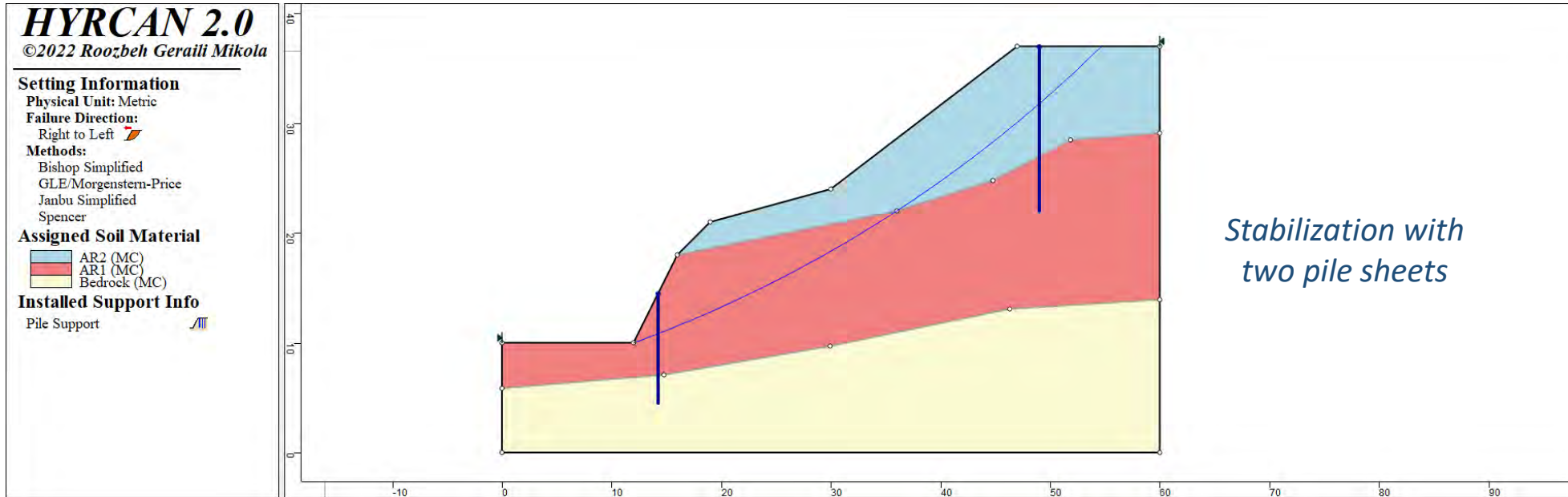
- 1.31E+01
- 1.15E+01
- 1.00E+01
- 8.48E+00
- 6.96E+00
- 5.43E+00
- 3.90E+00
- 2.38E+00
- 8.52E-01



Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018

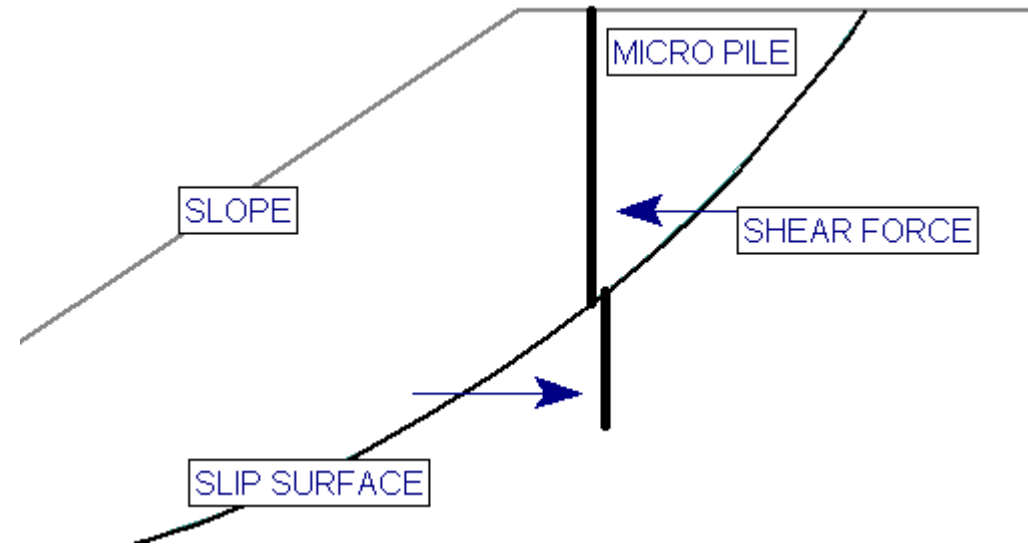
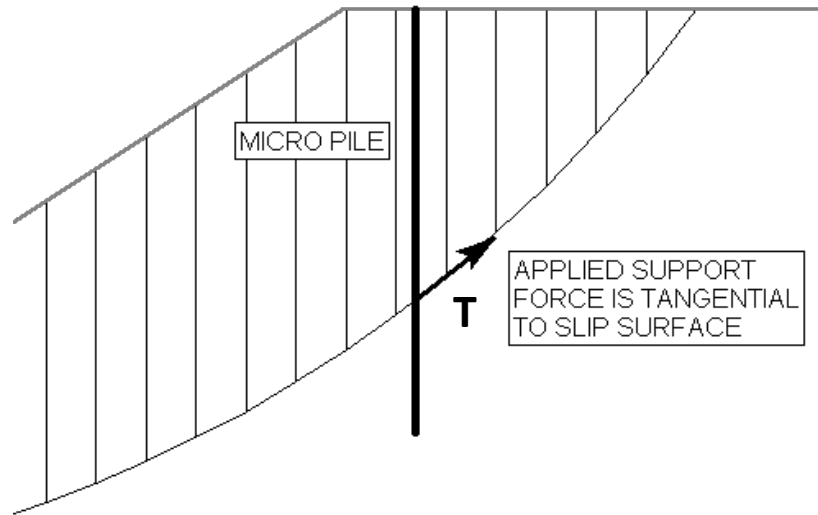


Case studies in Rwanda: RN 11 Kabaya – Ngororero, May 2018



Piles used as stabilising structures

Simplified modelling of resistance given by a pile as simulated in the software



Total shear resistance T (pile cross section):

$$T = \tau_{CLS} \cdot \pi \cdot r^2$$

$$\tau_{CLS} = 25000 \frac{kN}{m^2}$$



Pile diameter (2r):

30cm

30-80cm

>80cm

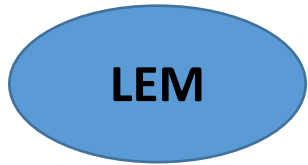
micropiles

medium diameter piles

large diameter piles

Summary

- **SINGLE SLOPE SCALE**



HYRCAN 2.0
©2022 Roozbeh Geraili Mikola



Landslide inventory (+ MAP):

- Date (day/month/year)
- Position (on the map)
- Shape (edge as polygon on the map)/2D profile
- Type
- Type of material (rock of soil)
- New or re-activated
- Land use
- Damage
- Photo

Data collection:

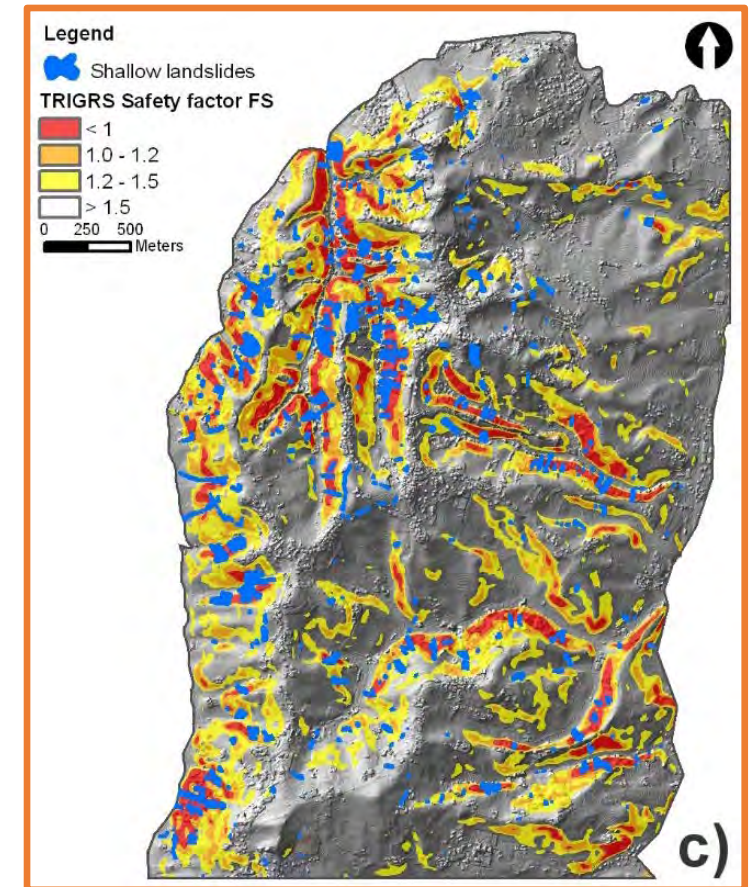
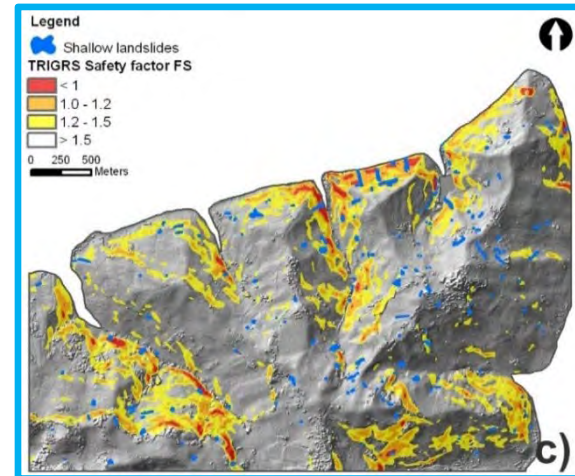
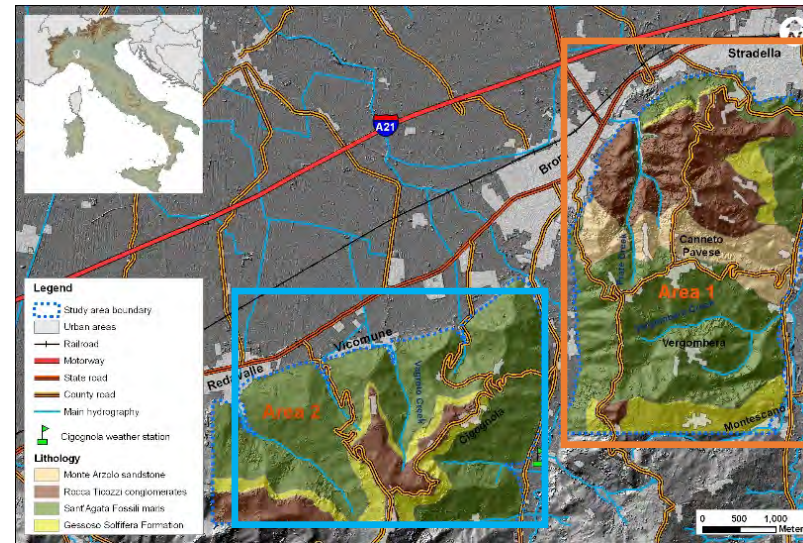
- ✓ Soil/rock samples
- ✓ Lab tests for classification and mechanical characterization

SLOPE STABILITY ANALYSIS AT LARGE SCALE



SLOPE STABILITY ANALYSIS AT LARGE SCALE

TRIGRS - Transient Rainfall Infiltration and Grid-Based Regional Slope-Stability Analysis (Baum et al., 2008)



TYPE OF LANDSLIDE: SHALLOW LANDSLIDES INDUCED BY RAINFALL



Alta Versilia – 19.06.1996 (*Tuscany*)



Sarno – 05.05.1998 (*Campania*)



Ceriana – 23.11.2000 (*Liguria*)



Oltrepo Pavese – 26.11.2002 (*Lombardy*)



App. Reggiano – 10.04.2005 (*Emilia Romagna*)



Giampilieri – 01.10.2009 (*Sicily*)



Casamicciola Terme – 10.11.2009 (*Campania*)

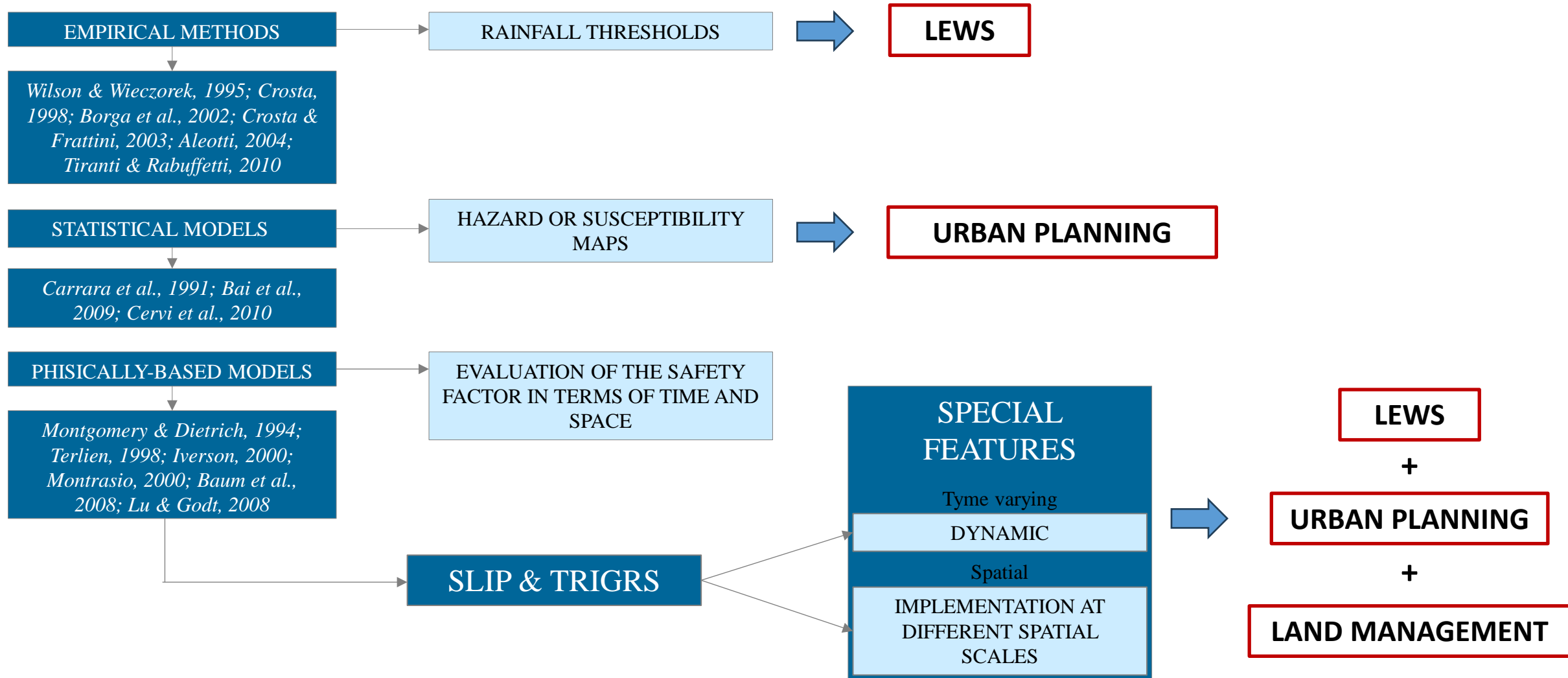


San Fratello – 14.02.2010 (*Sicily*)



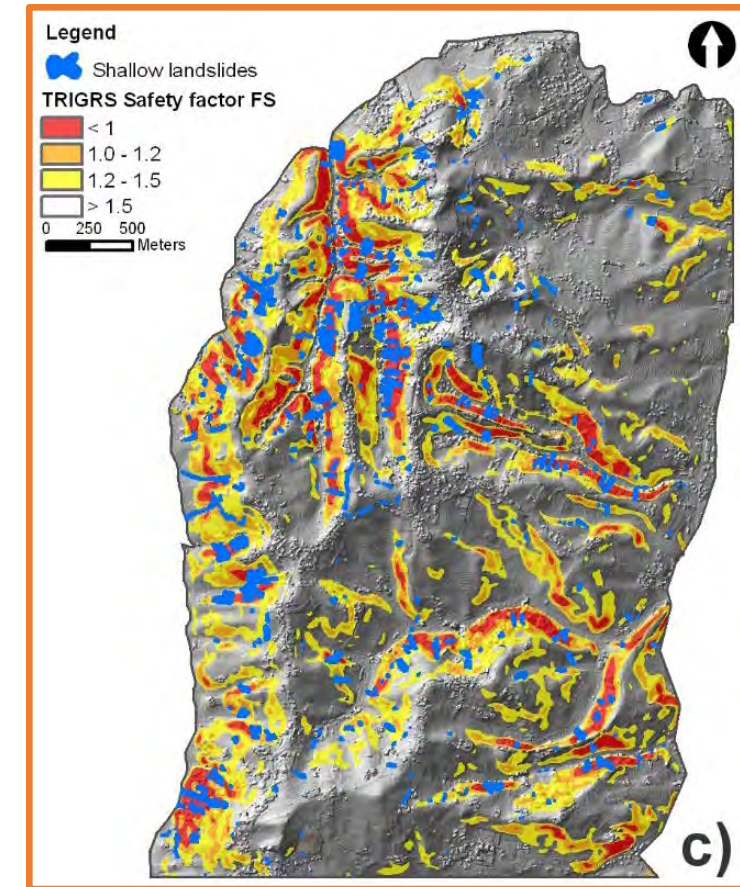
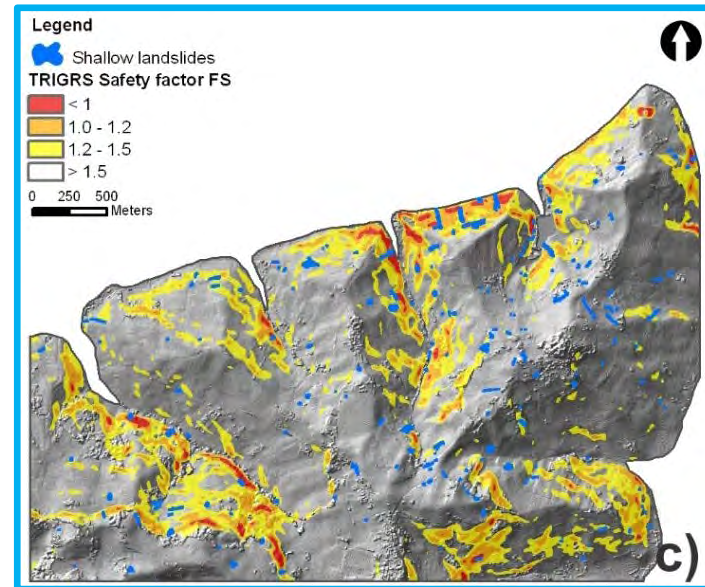
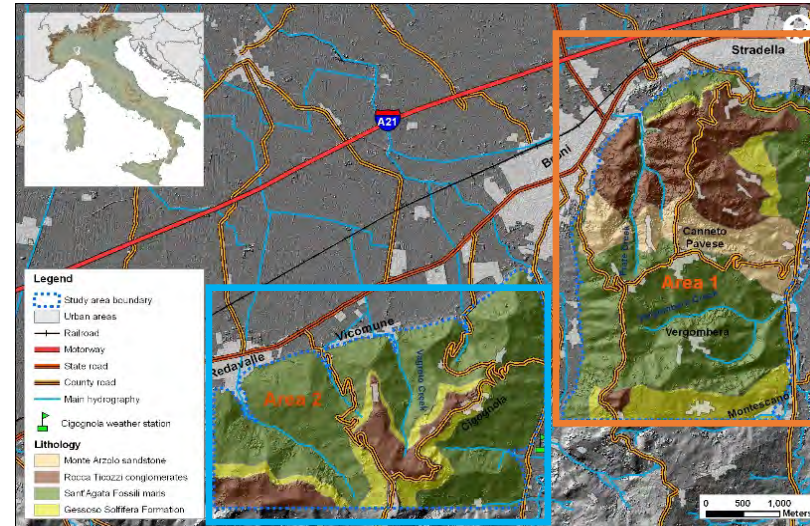
Massa Carrara – 01.11.2010 (*Tuscany*)

«DISTRIBUTED» SLOPE STABILITY ANALYSIS



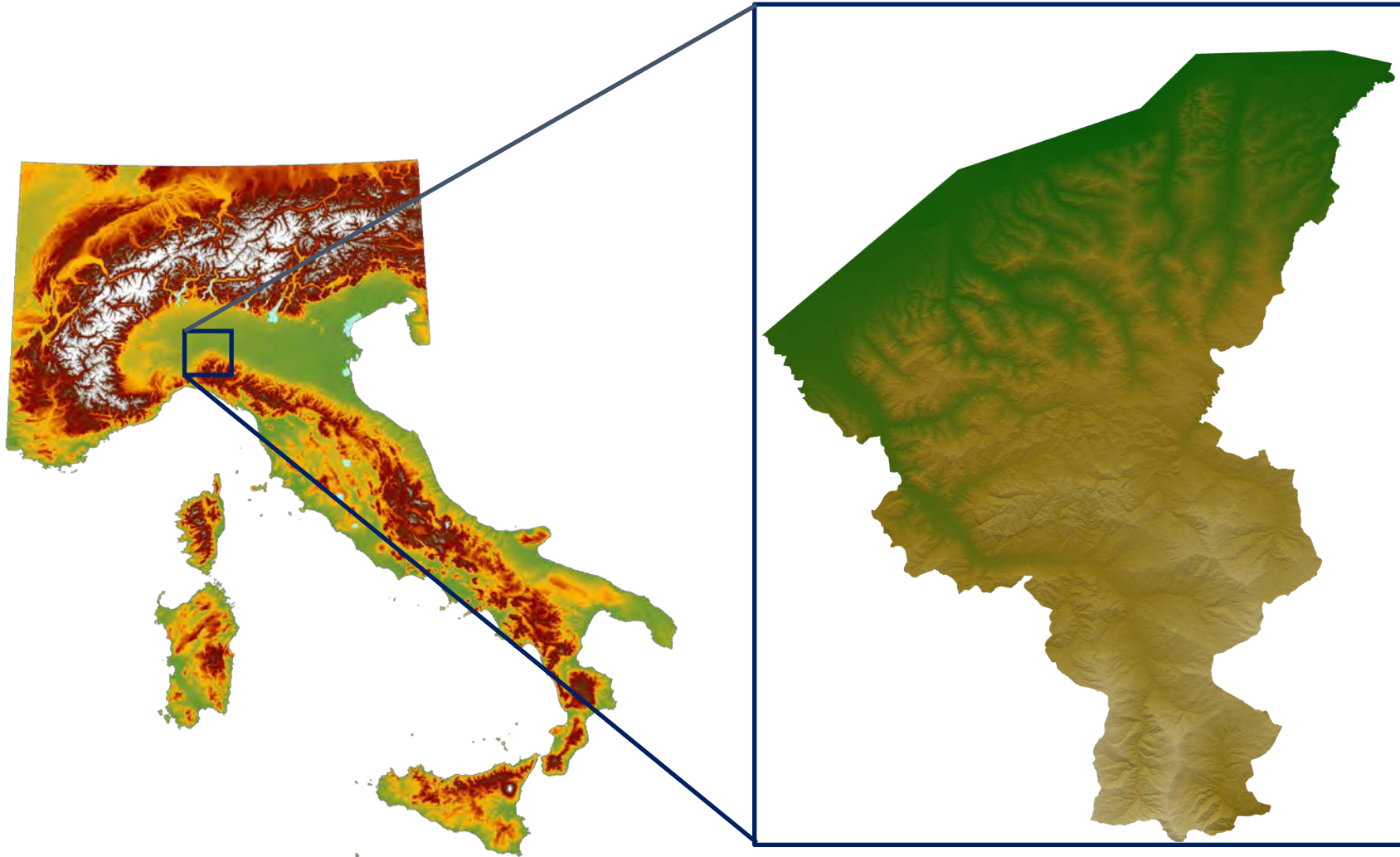
LARGE SCALE MODELING

TRIGRS - Transient Rainfall Infiltration and Grid-Based Regional Slope-Stability Analysis (Baum et al., 2008)



Case study in Italy: Oltrepò Pavese, April 2009

Oltrepò Pavese is in the north-west of Italy and lies to the south of the Po river.

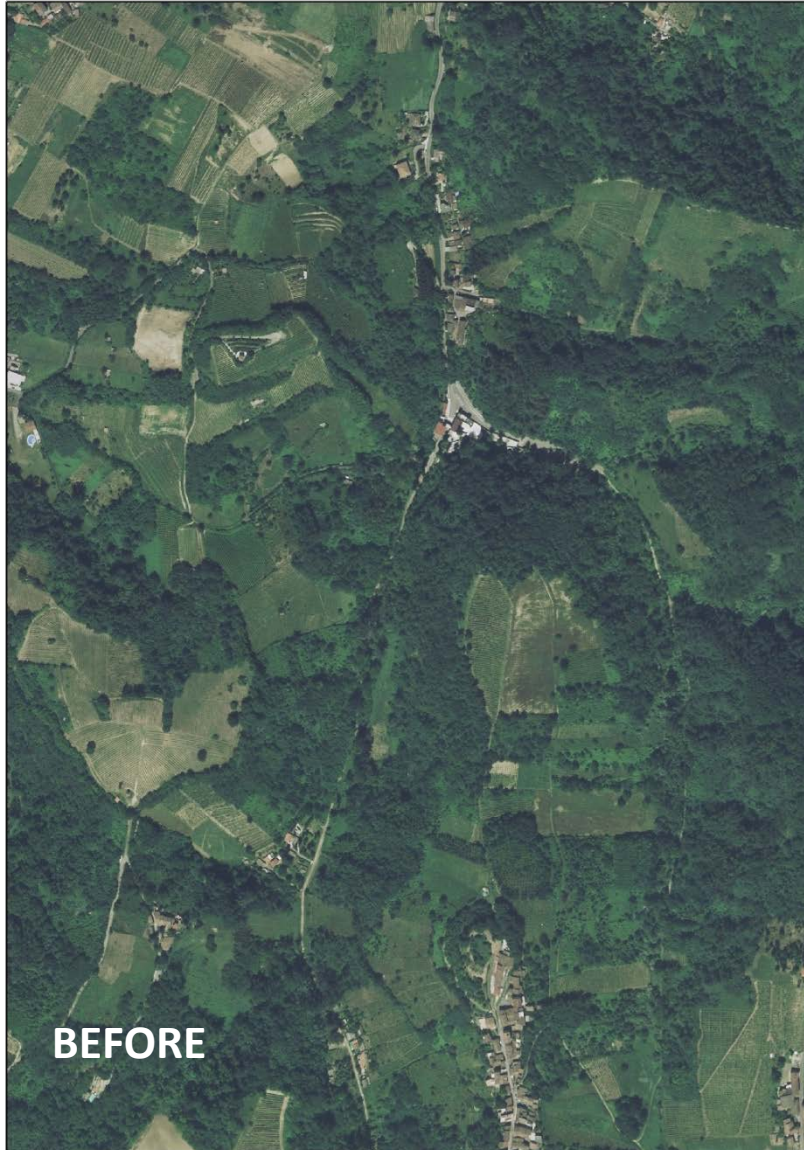


Case study in Italy: Oltrepò Pavese, April 2009



Case study in Italy: Oltrepò Pavese, April 2009

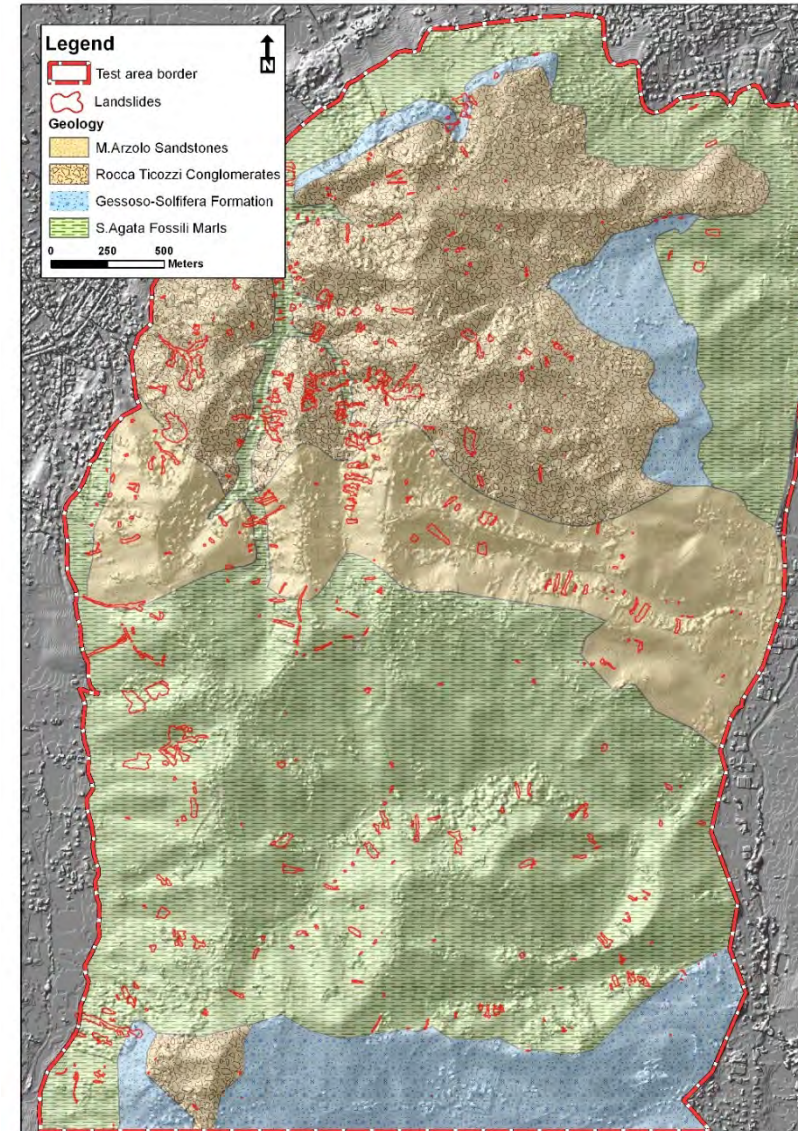
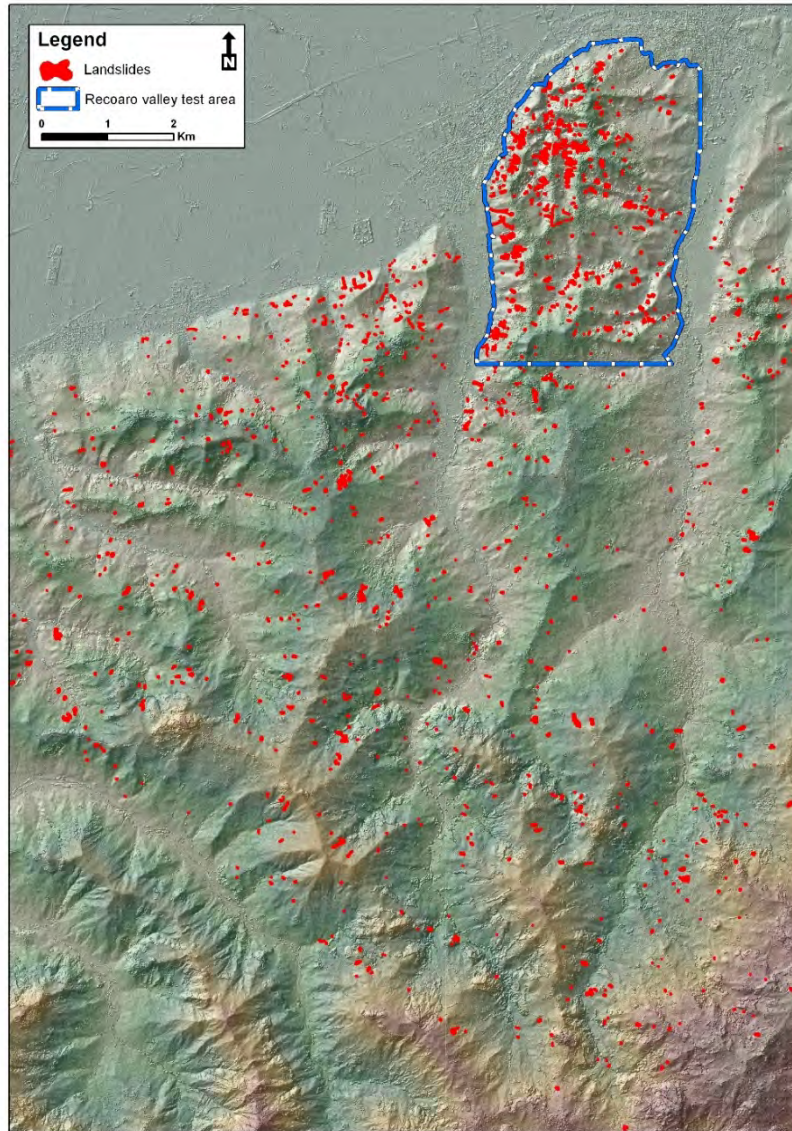
The Recoaro Valley was the most affected area by this event



Case study in Italy: Oltrepò Pavese, April 2009

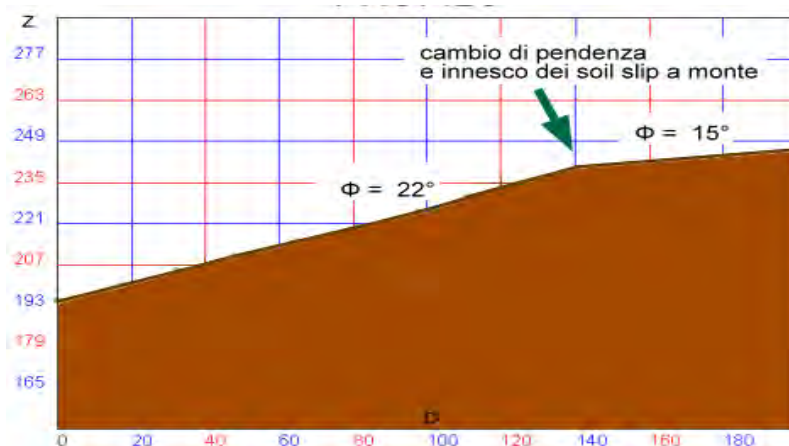
More than 1600
landslides in 180
km²

No. 491 landslides
in 17 km²



Case study in Italy: Oltrepò Pavese, April 2009

Predisposing factors



- Most of the landslides seems to be concentrated in areas where the slope angle changed from a gentle slope to a steep slope
- These areas correspond to morphological or anthropogenic slopes characterized by a greater thickness of the soil, often coincident with the passage vineyard-forest

Case study in Italy: Oltrepò Pavese, April 2009

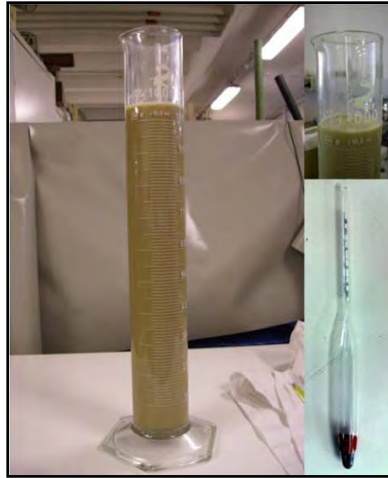
Collection of soil samples
(undisturbed and disturbed)



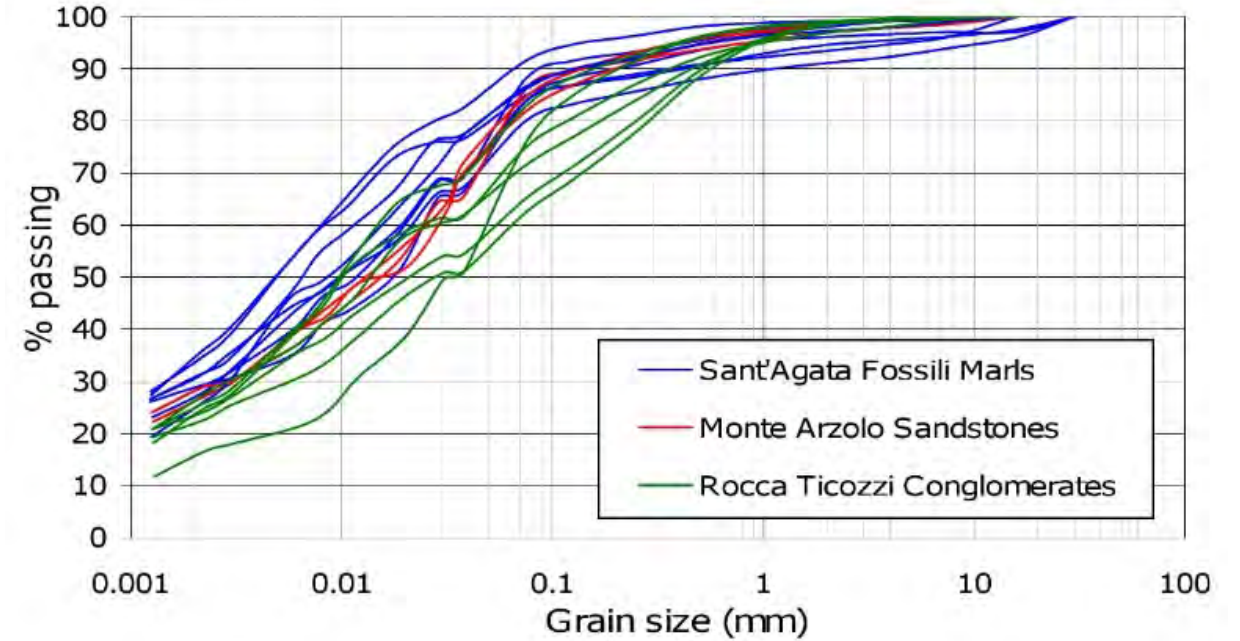
Case study in Italy: Oltrepò Pavese, April 2009

Soil classification

SEDIMENTATION



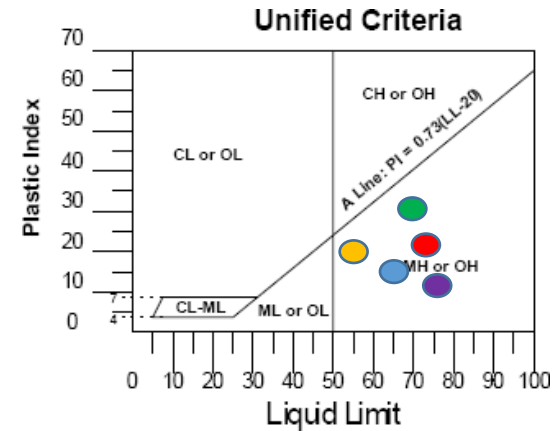
SIEVING



LIQUID LIMIT

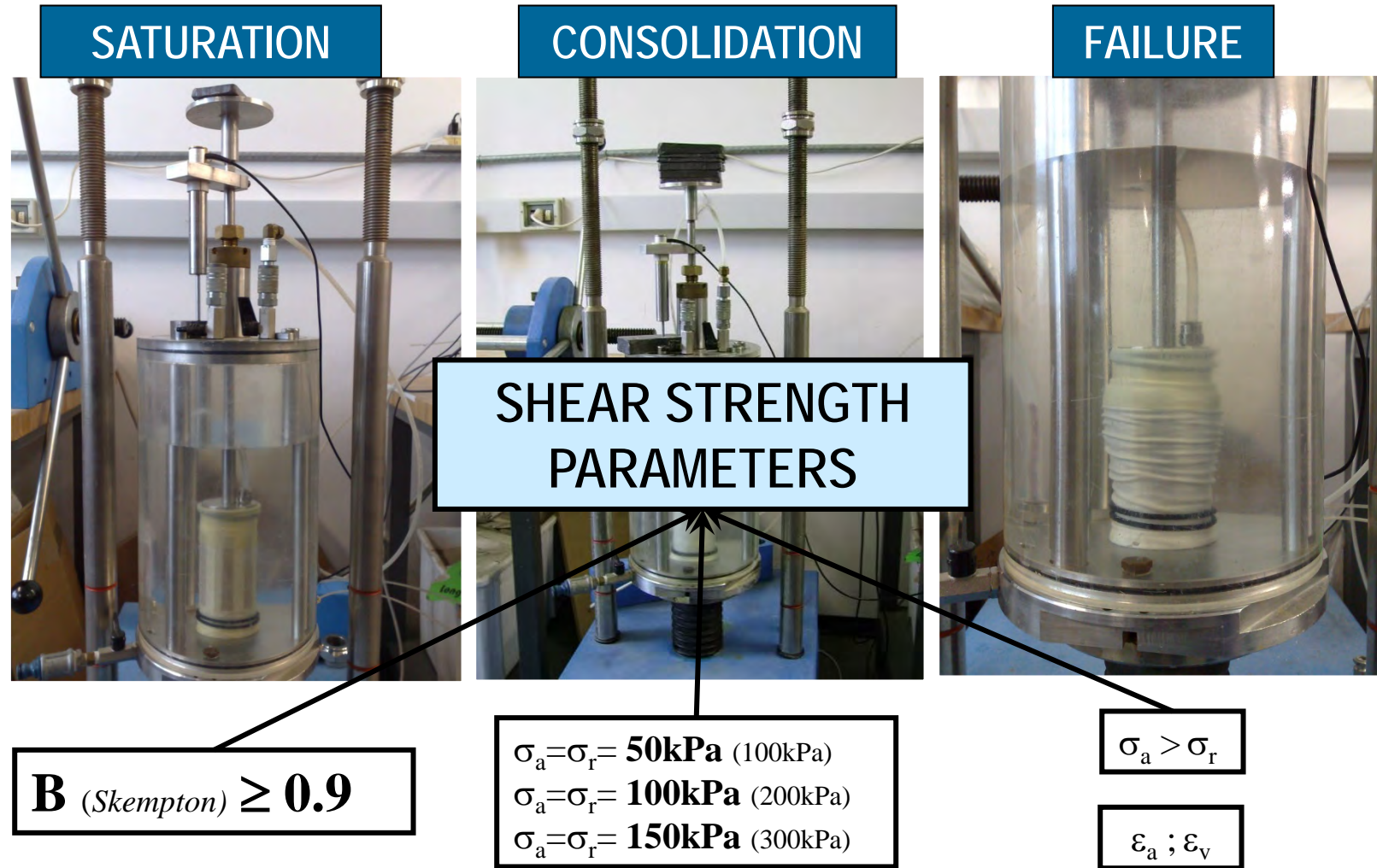


PLASTIC LIMIT



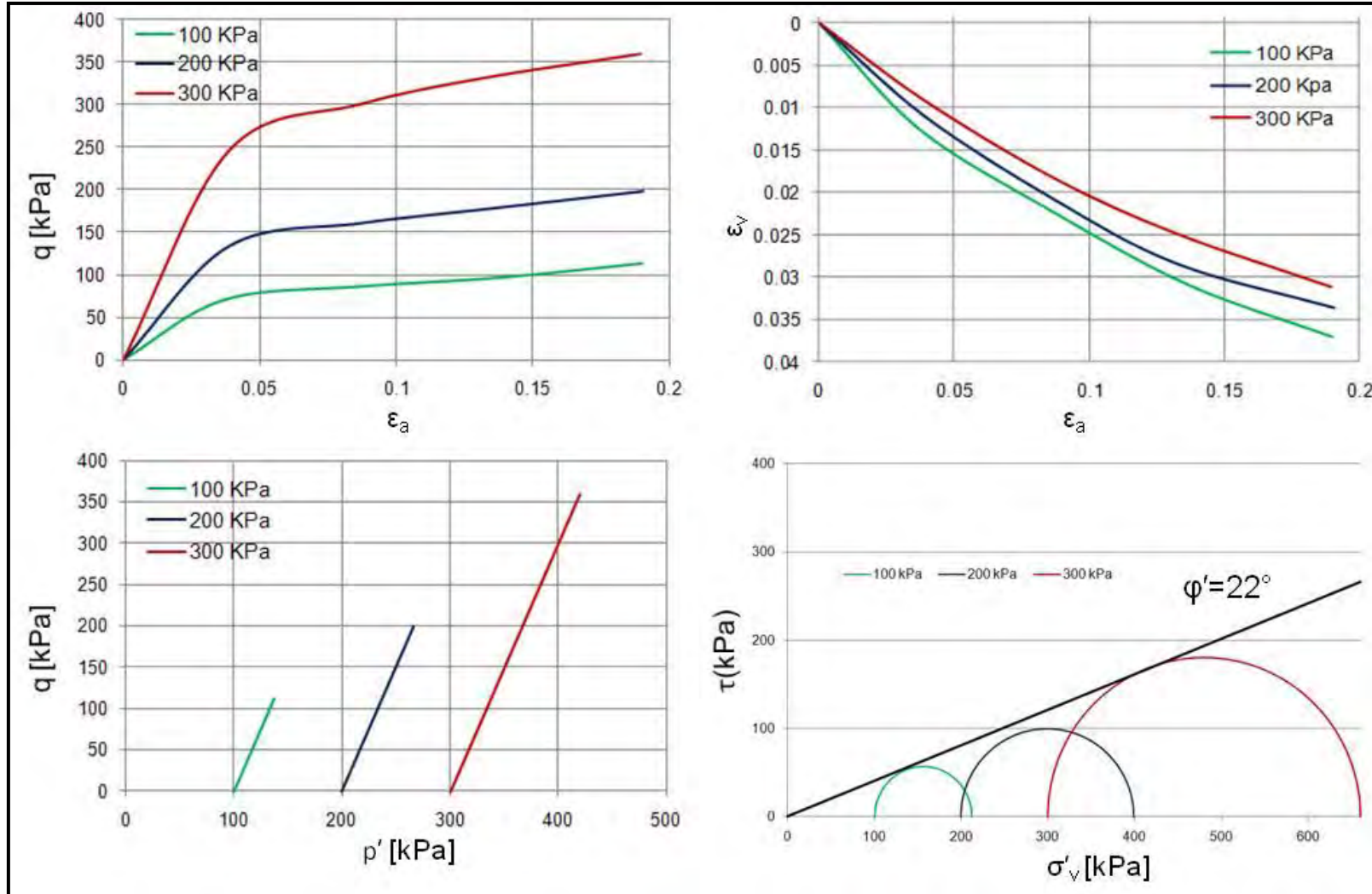
Case study in Italy: Oltrepò Pavese, April 2009

Soil mechanical characterization: triaxial tests



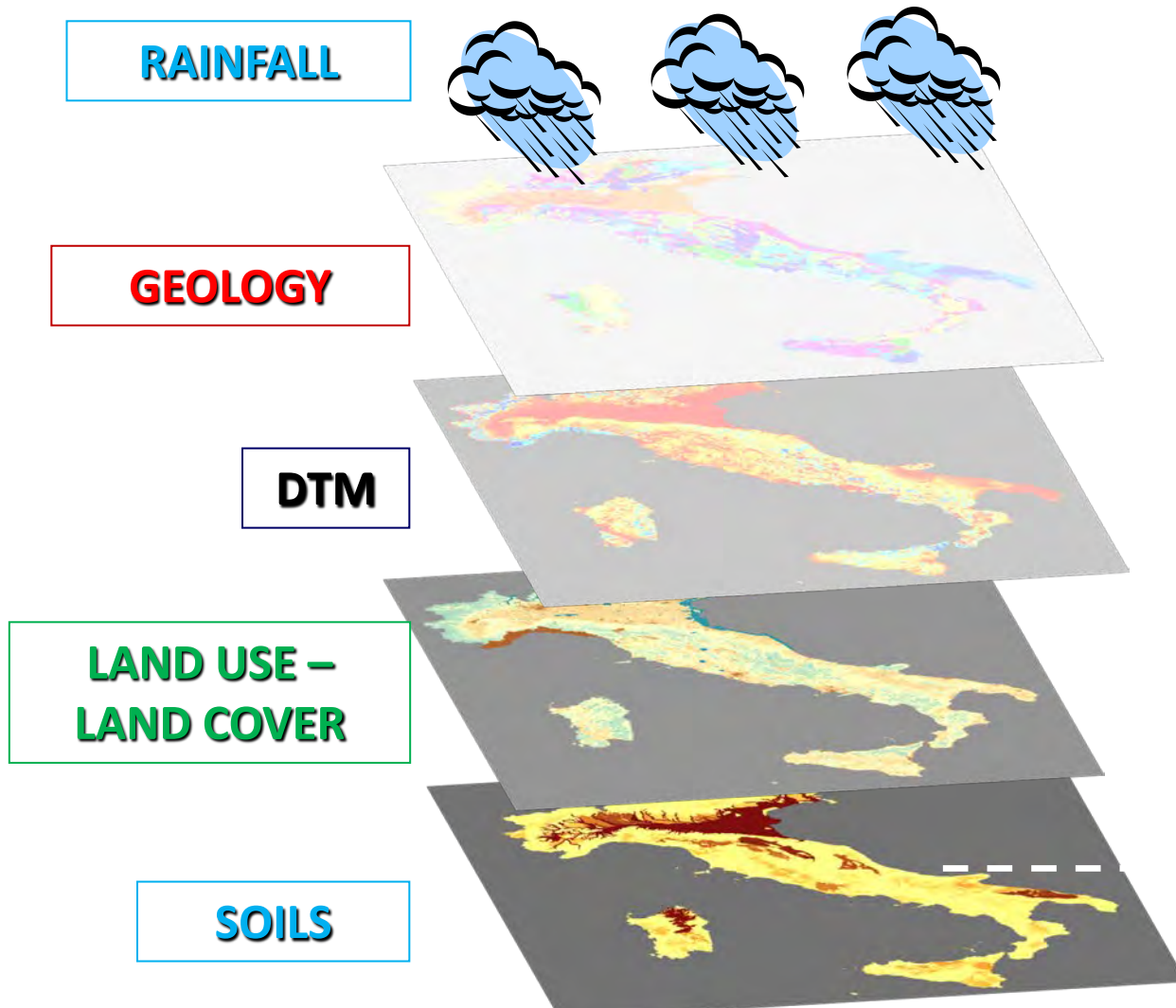
Case study in Italy: Oltrepò Pavese, April 2009

Soil mechanical characterization: results of triaxial tests



Case study in Italy: Oltrepò Pavese, April 2009

Implementation of a distributed slope stability model at regional scale



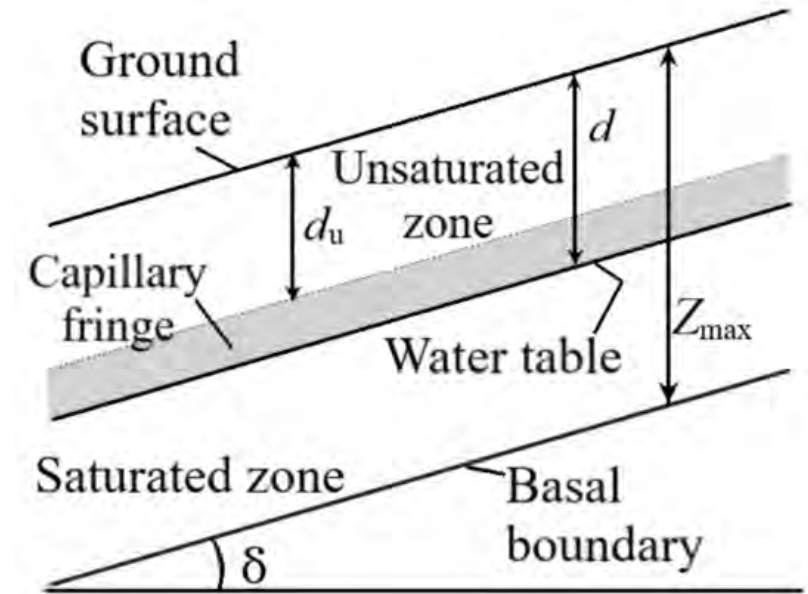
Pre-Processing:

Data at large scale are acquired at the maximum available resolution.

Use of GIS

Case study in Italy: Oltrepò Pavese, April 2009

- ❑ **TRIGRS** (Transient Rainfall Infiltration and Grid-based Regional Slope-stability analysis) model designed for modeling the timing and distribution of shallow, rainfall-induced landslides (Baum et al., 2002; Baum et al., 2008)
- ❑ The program computes transient pore-pressure changes, and attendant changes in the factor of safety, due to rainfall infiltration
- ❑ The program models rainfall infiltration, resulting from storms that have durations ranging from hours to a few days, using analytical solutions for partial differential equations that represent one-dimensional, vertical flow in isotropic, homogeneous materials for either saturated or unsaturated conditions
- ❑ TRIGRS uses an infinite-slope model to compute a factor of safety (FS) calculation for each grid cell.



$$Fs(Z;t) = \frac{\tan \varphi'}{\tan \beta} + \frac{c' - \psi(Z;t) \gamma_w \tan \varphi'}{\gamma_s Z \sin \beta \cos \beta}$$

Model inputs:

β = slope angle (input as grid)

φ' = friction angle of the soil

c' = effective cohesion

γ_s = soil unit weight

γ_w = water unit weight

Z = depth to bedrock (input as grid)

K_s = permeability

D = hydraulic diffusivity

Rainfall intensity and duration

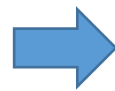
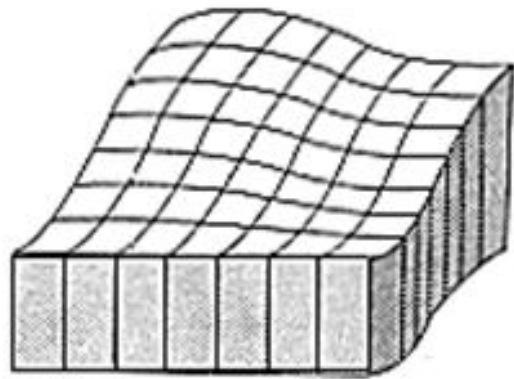
Water table depth (d)

Case study in Italy: Oltrepò Pavese, April 2009

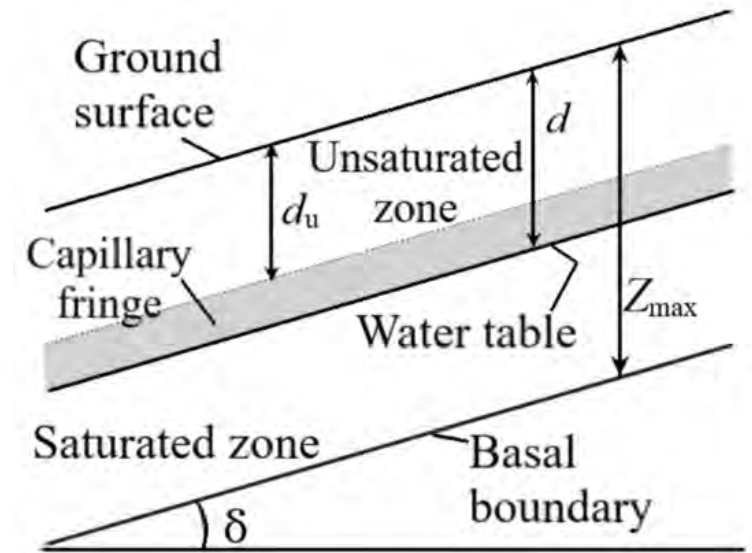
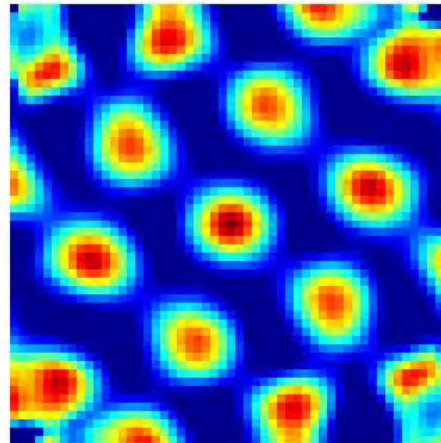
- TRIGRS uses an infinite-slope model to compute a factor of safety (FS) calculation *for each grid cell*.

$$F_s(Z;t) = \frac{\tan \varphi'}{\tan \beta} + \frac{c' - \psi(Z;t) \gamma_w \tan \varphi'}{\gamma_s Z \sin \beta \cos \beta}$$

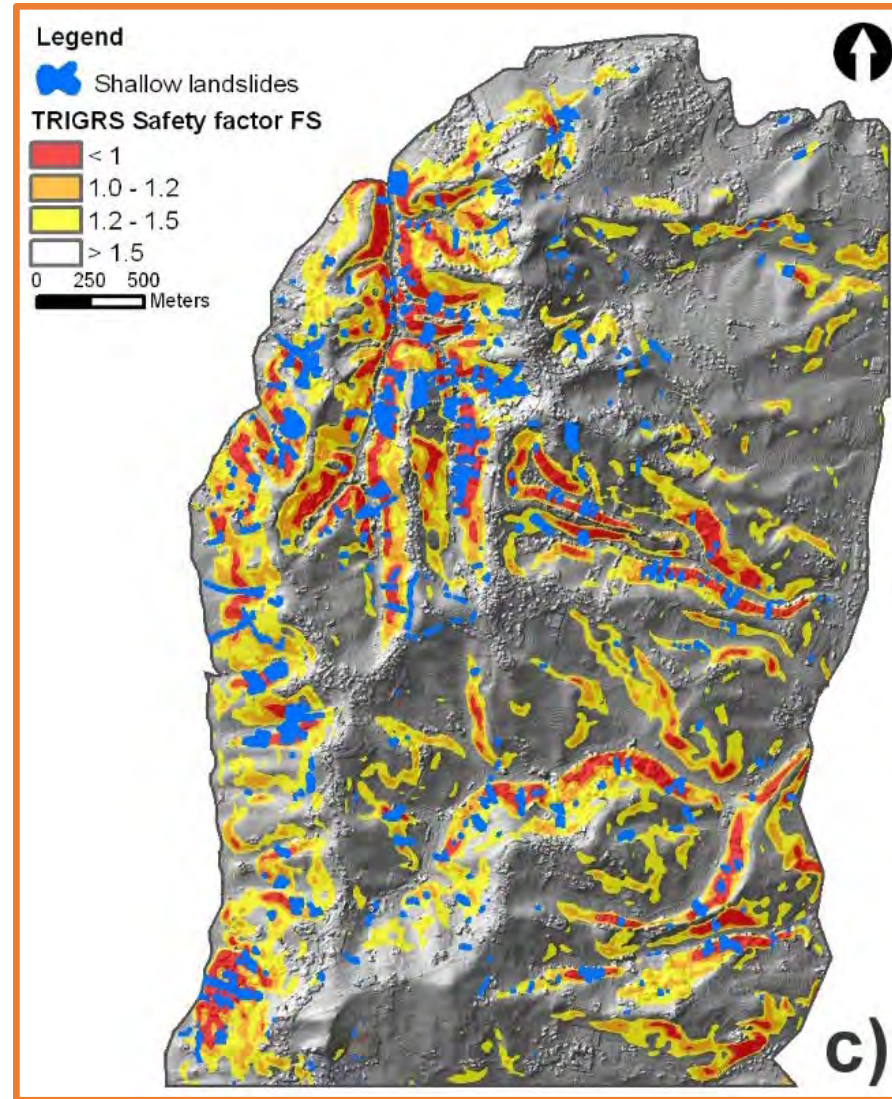
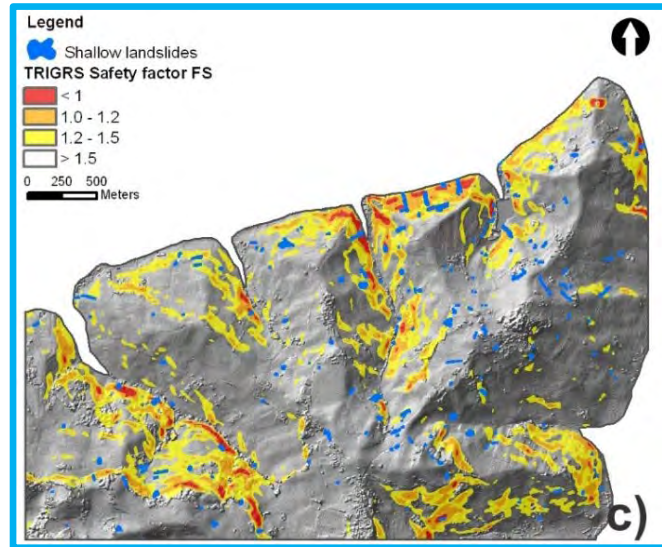
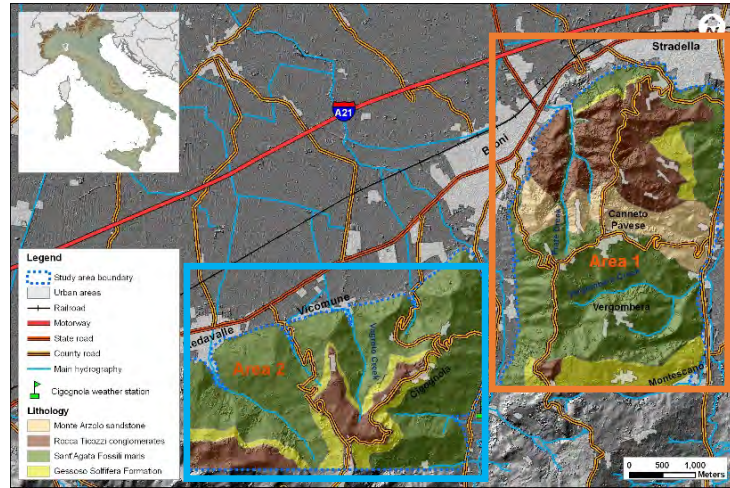
Elementary grid cell 10m x 10m



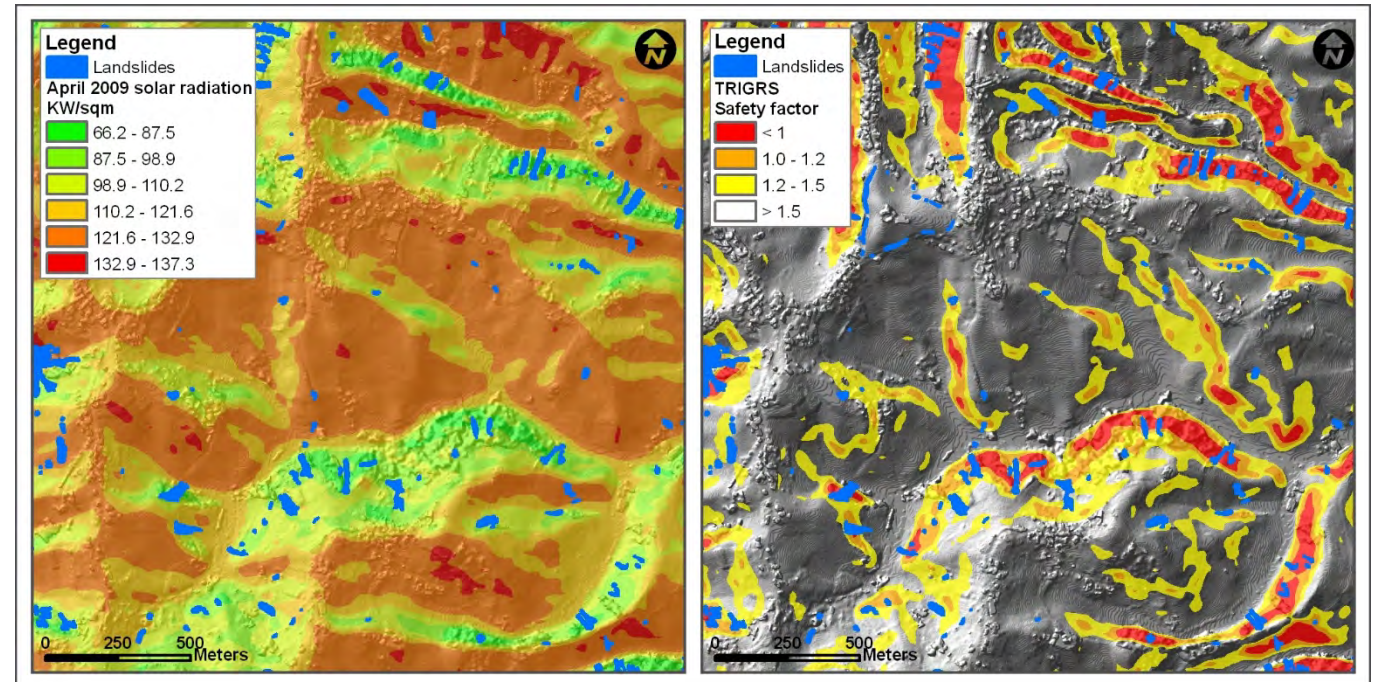
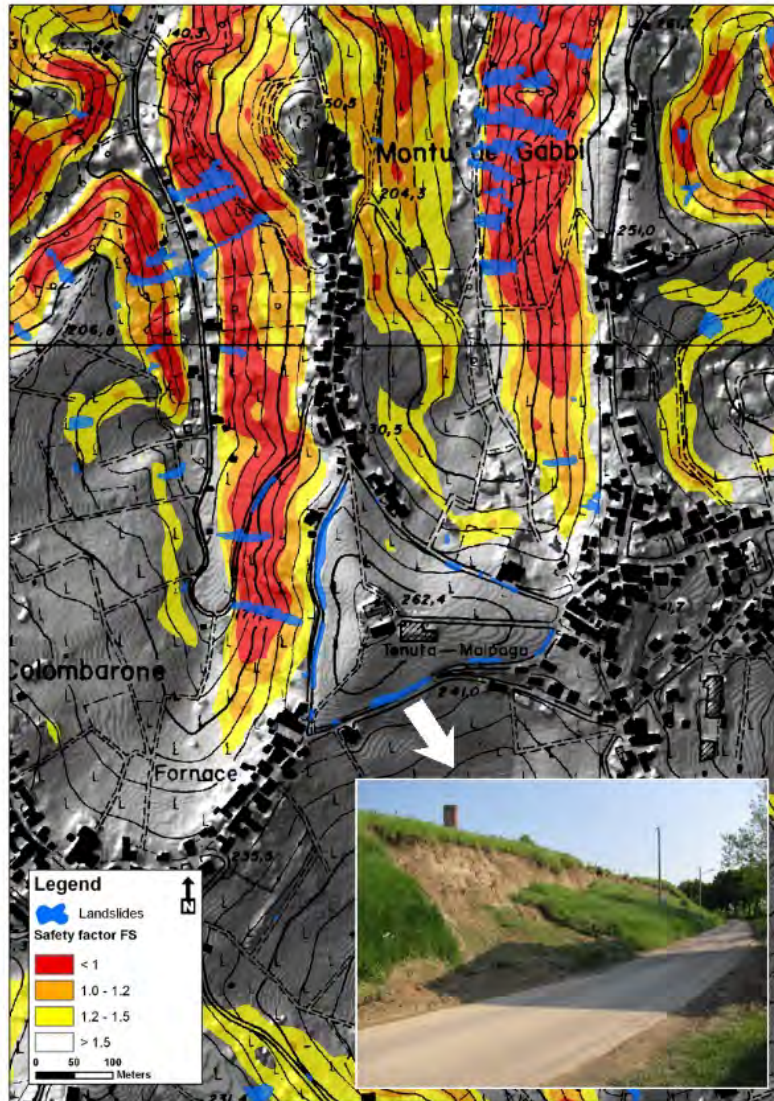
Fs map



Case study in Italy: Oltrepò Pavese, April 2009

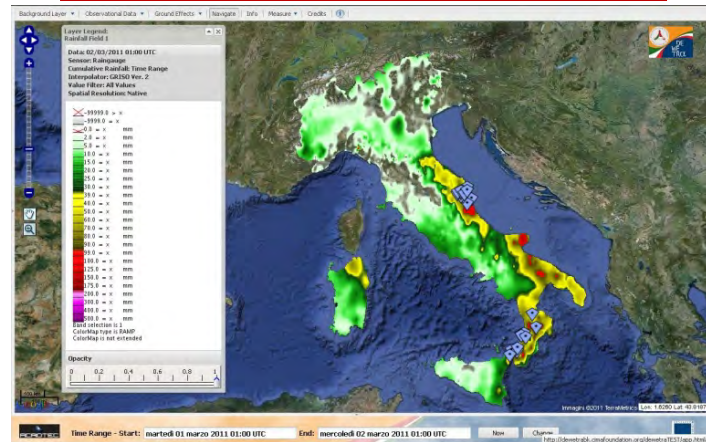


Case study in Italy: Oltrepò Pavese, April 2009



SLIP model at national scale in Italy

Rainfall map



Geospatial data



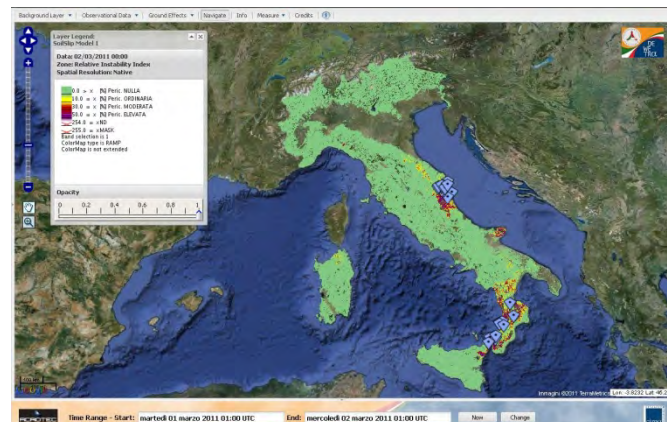
SLIP model

Safety Factor (F_s) on elementary cells 20m x 20m

DEWETRA
reference operative system aimed
to prevent, detect and mitigate
potential weather related risks



NATIONAL CIVIL
PROTECTION CENTRE



INSTABILITY INDEX
Ratio between number of
elementary cells having $F_s \leq 1$
and total cells on areas
1km x 1km

ALERT LEVELS

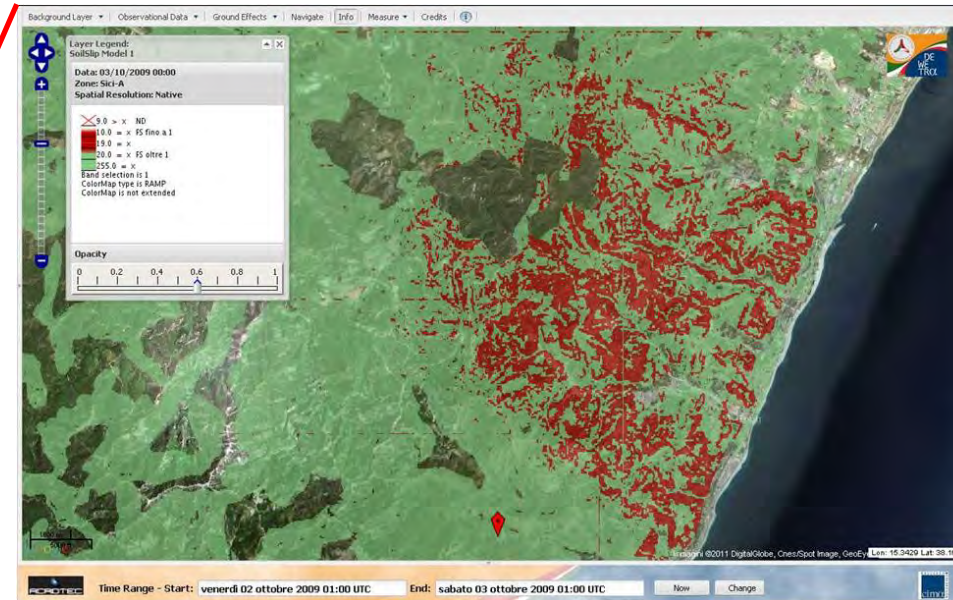
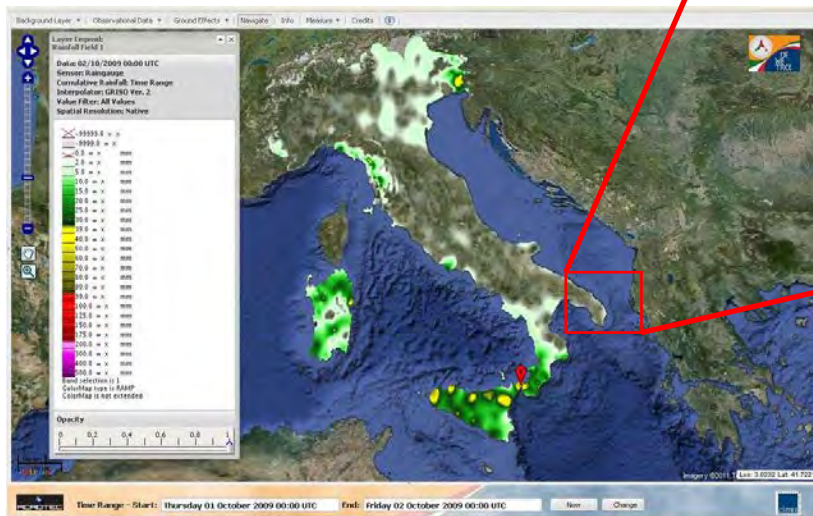
Low
Ordinary
Moderate
High

SLIP model at national scale in Italy

Event occurred at Giampileri (ME) on the 1th of October 2009 (37 victims)
 F_s map (20m x 20m)



Rainfall map



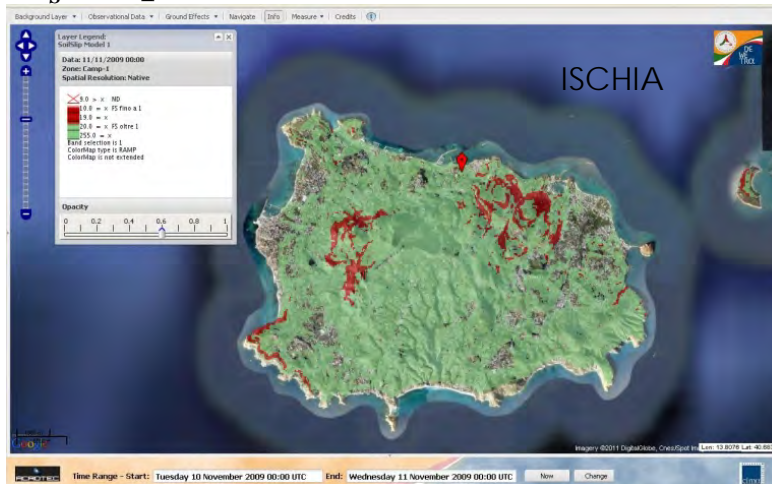
Instability index

SLIP model at national scale in Italy

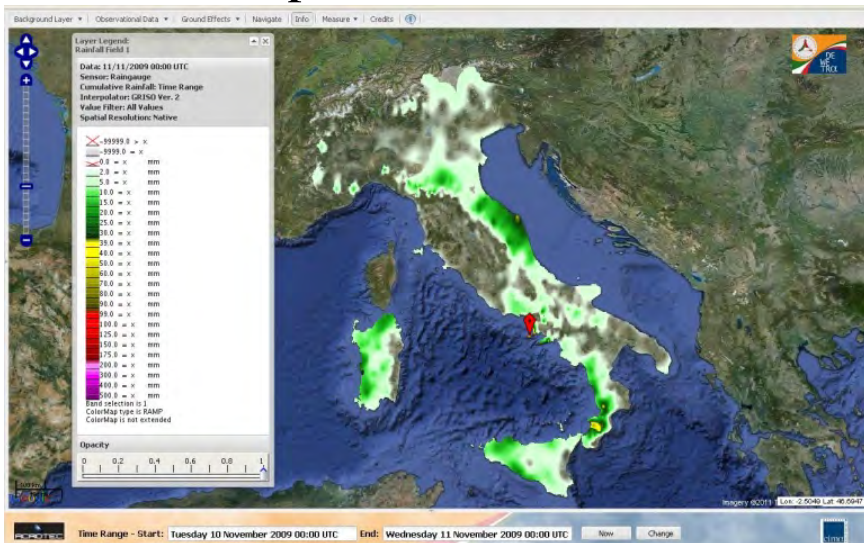
Event occurred at Casamicciola Terme (Isle of Ischia) on the 10th of November 2009 (1 victim)



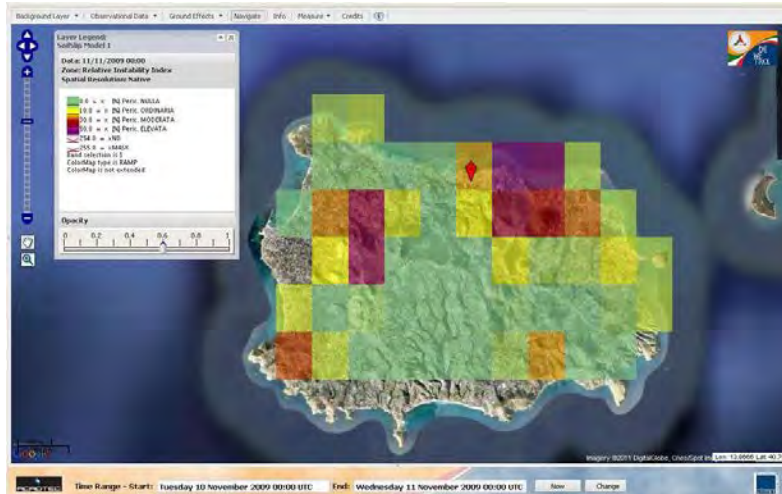
F_s map (20m x 20m)



Rainfall map



Instability index (1km x 1km)

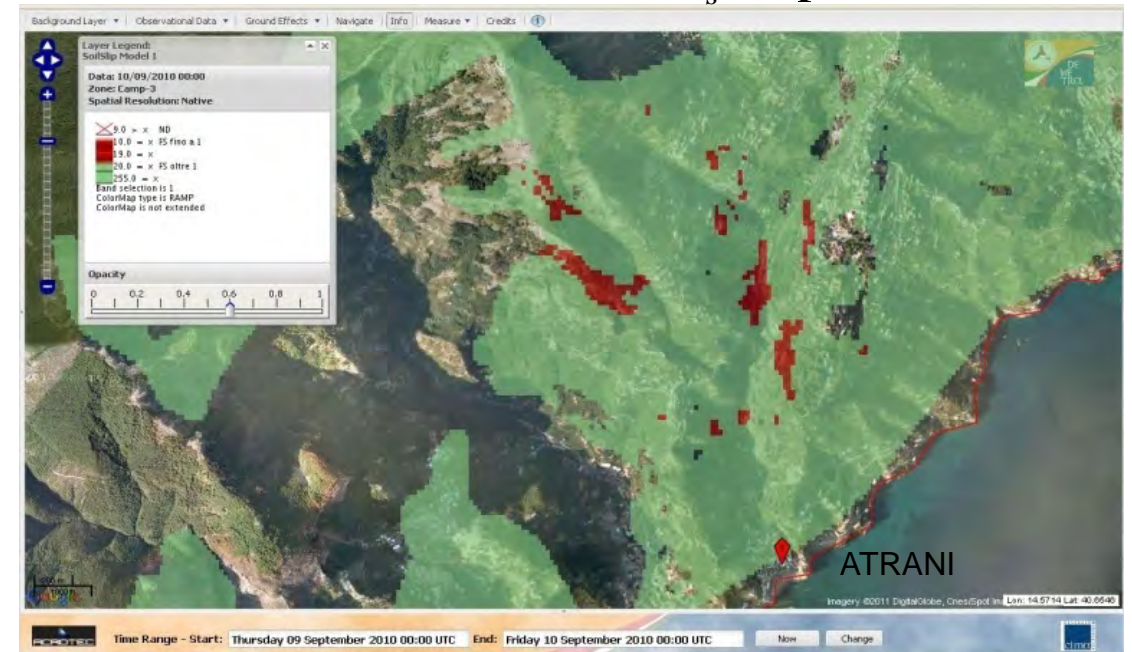


SLIP model at national scale in Italy

Event occurred at Atrani on 9th of September 2010 (1 victim)



F_s map (20m x 20m)



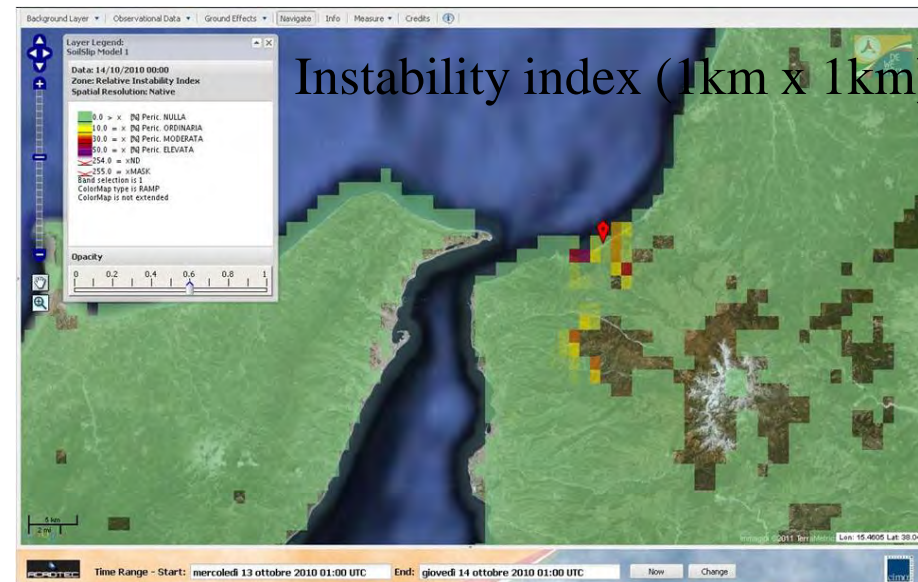
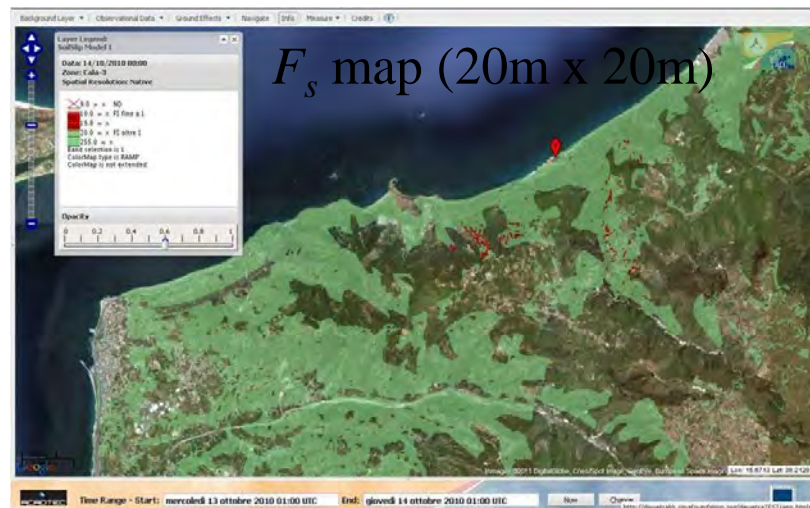
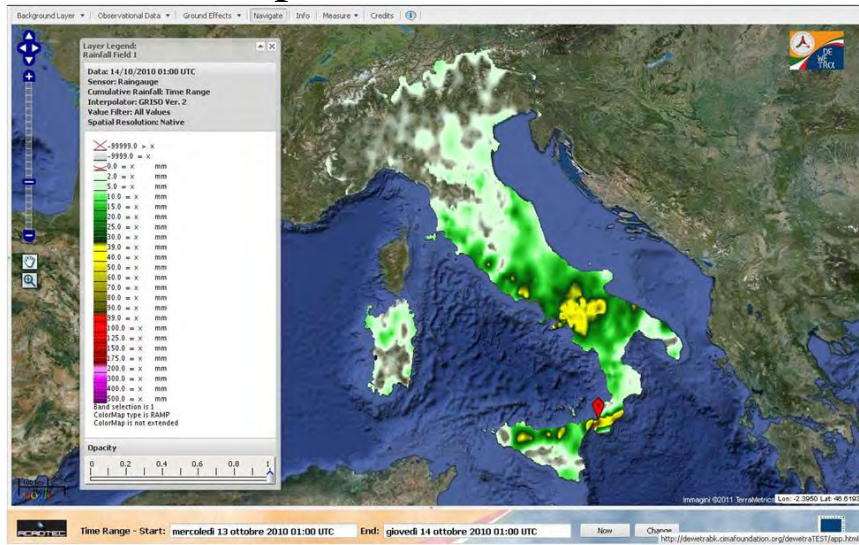
Zoom on the triggering areas



SLIP model at national scale in Italy

Event occurred at Reggio Calabria on the 13th of October 2010

Rainfall map



SLIP model at national scale in Italy

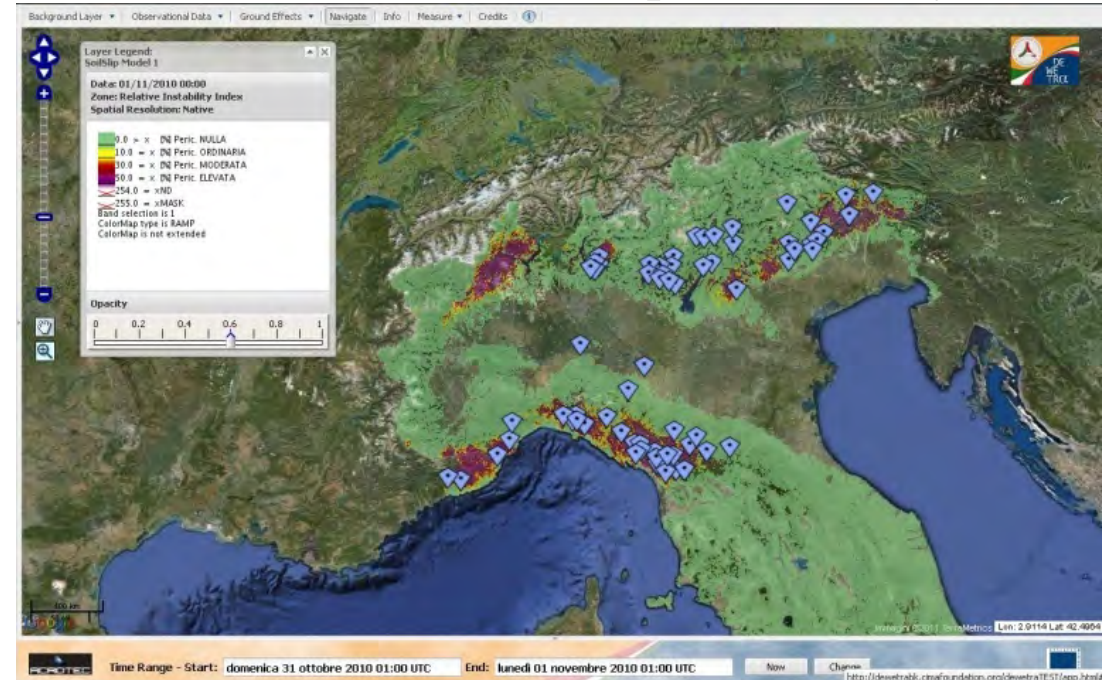
Event occurred in Northern Italy on the 31st of October 2010 (3 victims)



Rainfall map



Map of instability index

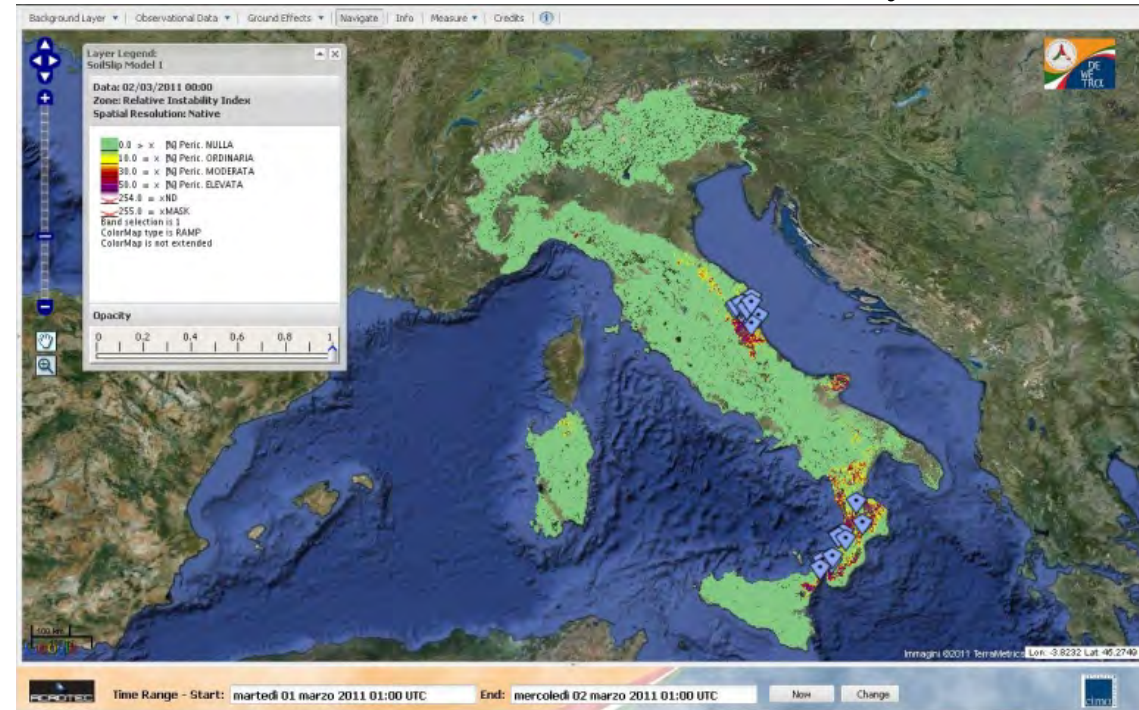


SLIP model at national scale in Italy

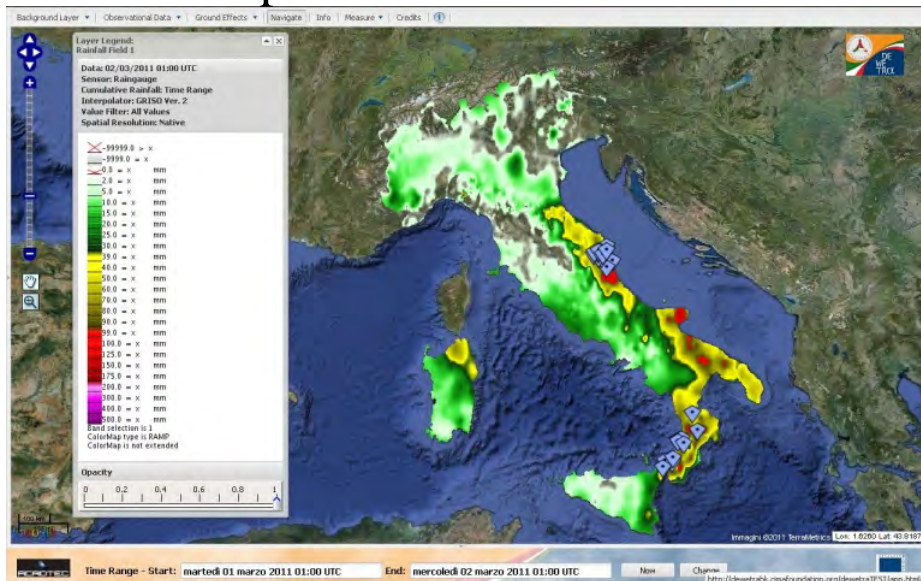
Events occurred on the 1st of March 2011 in Marche and Calabria



Instability index



Rainfall map



Summary

LEM

- SINGLE SLOPE SCALE



HYRCAN 2.0
©2022 Roozbeh Geraili Mikola



- LARGE SCALE

TRIGRS
SHALSTAB
SINMAP
SLIP

Landslide inventory (+ MAP):

- Date (day/month/year)
- Position (on the map)
- Shape (edge as polygon on the map)/2D profile
- Type
- Type of material (rock of soil)
- New or re-activated
- Land use
- Damage
- Photo

Data collection:

- ✓ Soil/rock samples
- ✓ Lab tests for classification and mechanical characterization

THANK YOU
